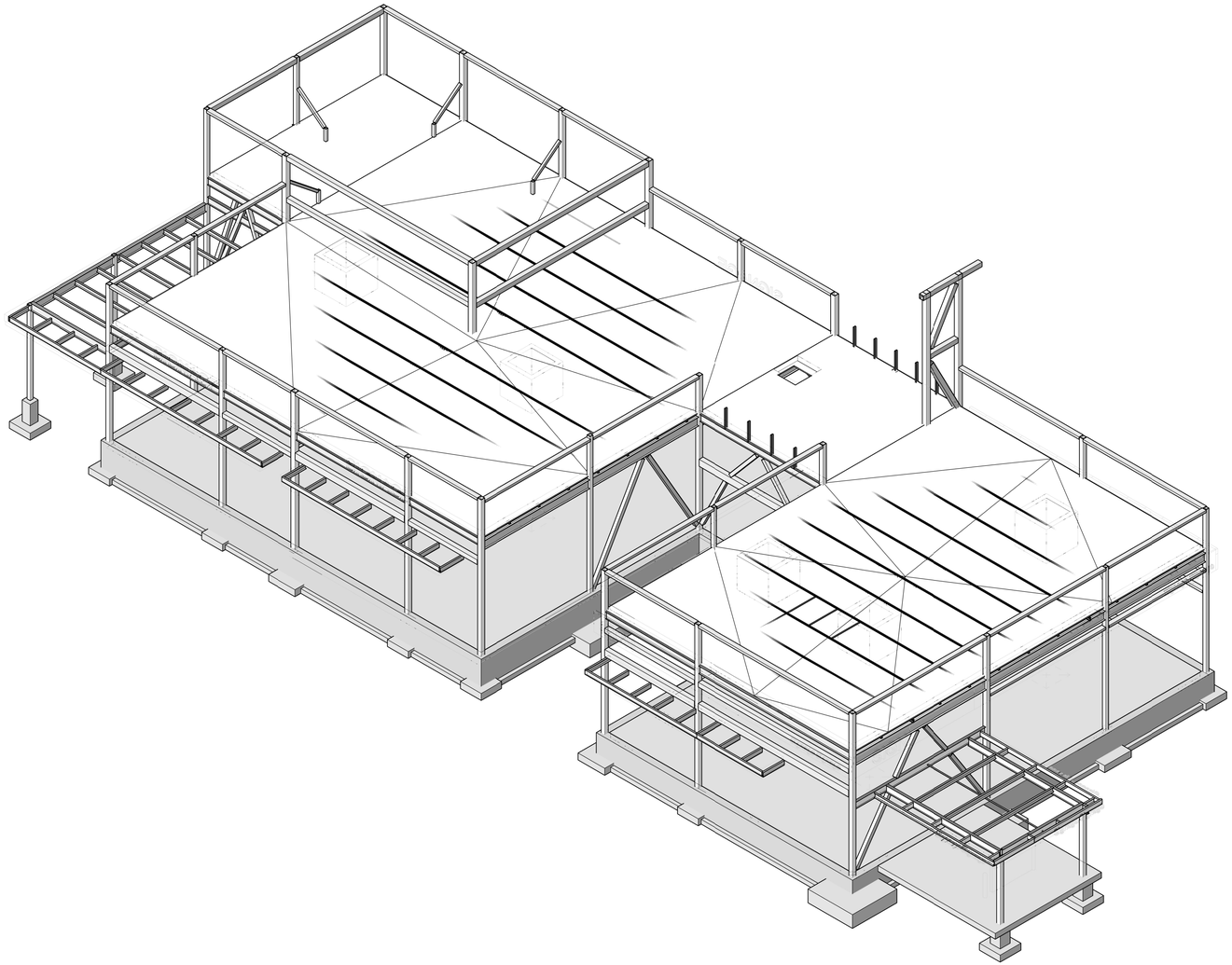
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS

STRUCTURAL DRAWINGS

PROJECT #22-3849

BLOCK C2 PROPOSED BUILDING C5

2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4



DWG NO.	DRAWING TITLE
S-00-C5	COVER
S-01.1-C5	GENERAL NOTES AND TYPICAL DETAILS
S-02.1-C5	FOUNDATION PLAN
S-02.2-C5	FOUNDATION TYPICAL DETAILS
S-03.1-C5	ROOF FRAMING PLAN
S-03.2-C5	CANOPY FRAMING PLAN
S-04.1-C5	BUILDING ELEVATIONS
S-04.2-C5	BUILDING ELEVATIONS
S-05.1-C5	SECTIONS
S-05.2-C5	SECTIONS
S-05.3-C5	SECTIONS
S-05.4-C5	SECTIONS

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY

REVISION

LICENSED PROFESSIONAL ENGINEER
L.KALISHENKO
16/06/2023
PROVINCE OF ONTARIO

LEONARD KALISHENKO & ASSOCIATES LIMITED
STRUCTURAL ENGINEERS
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5050 DUFFERIN ST.#240 TORONTO, ON, M3H 5T5

PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
COVER

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-00-C5**

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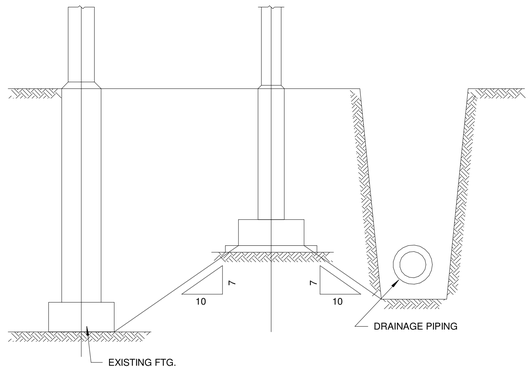
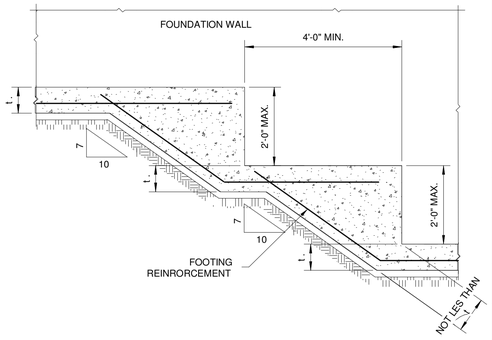


EXAMINE DRAINAGE INVERTS PRIOR TO CONSTRUCTION OF FOOTINGS. FOOTINGS IN THE VICINITY OF DRAINS ETC. SHALL BE LOWERED TO SUIT MAX. SLOPE OF 7 IN 10.

ELEVATIONS OF ADJACENT FOOTING EXCAVATION



TYPICAL DETAIL OF STEPPED FOOTING

INSPECTIONS:

THE FOLLOWING ITEMS REQUIRE TESTING & INSPECTIONS BY CERTIFIED INDEPENDENT TESTING & INSPECTION AGENCY UNLESS NOTED OTHERWISE. THE AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING & INSPECTION REPORTS TO ENGINEER FOR REVIEW:

ITEM	COMMENTS
SOIL BEARING CAPACITY	BY SOIL CONSULTANT
SOIL COMPACTION	
RE-BARS PLACEMENT	INSPECT FINAL PLACEMENT BEFORE CONCRETE POUR
CONCRETE COMPRESSIVE STRENGTH	MINIMUM 2 SETS PER 100 CUBIC METRES
NON-SHRINK GROUT COMPRESSIVE STRENGTH	3rd PARTY REVIEW
STRUCTURAL STEEL & JOISTS INSTALLATION	3rd PARTY REVIEW
STEEL DECK INSTALLATION	3rd PARTY REVIEW
STEEL STUDS INSTALLATION	BY STUD SUPPLIER ENGINEER

DESIGN CRITERIA:

- LIVE LOAD - SEE ROOF FRAMING PLAN NOTES
- DEAD LOAD - SEE ROOF FRAMING PLAN NOTES
- SNOW LOAD:
  - SEE ROOF FRAMING PLAN NOTES
  - Ss = 1.9kPa, Sr = 0.4 kPa
- WIND LOAD:
  - HOURLY WIND PRESSURE (1/50) q = 0.48 kPa
  - Ce, Cg, Cp, Iw FACTORS AS PER OBC 2012 REQUIREMENTS.

BUILDING LAYOUT

WIND	Y-Y	X-X
BASE SHEAR (kips)	23	51
OVERTURNING MOMENT (kip*ft)	414	918
DRIFT	1/500	1/1333

- SEISMIC LOAD:
  - Sa(0.2) = 0.192; Sa(0.5) = 0.108; Sa(1.0) = 0.058; Sa(2.0) = 0.029; Sa(5.0) = 0.0071; Sa(10.0) = 0.003; PGA=0.122; PGV =0.086;
  - Rd = 1.5; Ro = 1.3
  - SITE CLASS 'D', Fa=1.3; Fv=1.4
  - Ie = 1.0
  - Ta = 0.14sec
  - Period used:Ta=0.29 S(Ta)=0.24 (ETABS ANALYSIS)
  - Mv = 1.0
  - TORSIONAL SENSITIVITY = 2.0 > 1.7
  - TYPE OF IRREGULARITIES: IeFaSa(0.2) = 0.24 < 0.35 (Equivalent Static Force Procedure has been used in compliance with OBC 2012 Clause 4.1.8.7. (1),(a))
  - GOVERNING BASE SHEAR: Vmax = (2/3)S(0.2)IeW/(RdRo) = 0.08W
  - WEIGHT OF BUILDING W=1500kN=338kips

SEISMIC	Y-Y	X-X
BASE SHEAR (kips)	28	28
OVERTURNING MOMENT (kip*ft)	504	504
DRIFT	1/1150	1/2500

-SEISMIC FORCE RESISTING SYSTEM (SFRS) IS CONVENTIONAL CONSTRUCTION OF BRACED FRAMES.

-THE SFRS DIAPHRAGMS & THEIR CONNECTIONS HAVE BEEN DESIGNED IN COMPLIANCE WITH CAN/CSA S16-01 CLAUSE 27.10 & OBC 2012 ARTICLE 4.1.8.15.

-THE SFRS BRACED FRAMES & THEIR CONNECTIONS HAVE BEEN DESIGNED IN COMPLIANCE WITH CAN/CSA S16-01 CLAUSE 27.10.

-CONNECTION DETAILS FOR ALL MECHANICAL EQUIPMENT: PIPES, DUCTS, ROOF TOP UNITS ETC. TO BE DESIGNED BY MECHANICAL ENGINEER FOR SEISMIC FORCES IN ACCORDANCE WITH DIVISION B, ARTICAL 4.1.8.17 OF OBC 2012.

- SOIL BEARING PRESSURE: SEE FOUNDATION PLAN NOTES.
- EARTH LATERAL PRESSURE COEFFICIENTS:
  - Ka = 0.3,
  - Kp = 3.33
- WEIGHT OF BACKFILL = 20.5 kN/m3
- SOIL BEARING RESISTANCE: SEE FOUNDATION PLAN NOTES & SEE SOIL REPORT.

GENERAL NOTES:

1. Governing code - Ontario Building Code 2012 (R2020).
2. All dimensions and details given on Structural Drawings must be checked with the Architect's Drawings and any inconsistency must be reported to the Structural Engineer before proceeding with the work.
3. Drawings must not be scaled.

STRUCTURAL STEEL

1. All fabrication and erection to conform to CAN/CSA S16-19 latest edition
2. All structural steel to be Grade 350W.
3. Steel beams bearing on masonry walls shall have min. bearing of 6" unless otherwise noted on Plan.
4. Provide holes where required for the attachement of other materials.
5. Provide wall anchors for ends of all beams bearing on masonry.
6. Provide adjustable anchors @ 16" max. vertically for all columns in contact and adjacent with masonry.
7. All structural steel Shop Drawings to be submitted to the Structural Engineer for review and approval before proceeding with work.
8. Joists material and construction shall conform to current edition of The Ontario Building Code.
9. Joists design details, calculations and shop drawings, including welding details and cambers, etc. stamped and signed by a P. Engineer of Ontario, to be submitted to the design Engineer for review and approval.
10. Minimum bearing for joists to be 2 1/2" on steel beams and 4" on masonry or concrete.
11. Do not connect any structural members, piping or equipment to chords of joists between panel points unless chords have been designed for extra stress or an additional diagonal has been inserted at the point of connection.
12. Provide ceiling extension for joists where required by Architects.
13. Shop details and connection calculations, bearing stamp of a registered Professional Engineer, to be submitted to the Design Engineer for review and approval before proceeding with the fabrication as requested.
14. Do not put holes in top flanges of beams, where they cantilever over columns.
15. All bridging shall be completely installed before any construction loads are placed on joists.
16. All joists to be designed for loads shown on structural drawings.
17. All field bolts shall be ASTM A325 high strength bolts. Anchor bolts to be ASTM A307.
18. All welds shall conform to CSA Standards W59.
19. UNDERSIDE OF COLUMN BASE PLATE SHOULD BE AT -8" BELOW FINISHED FLOOR (TYPICAL U/N) & AT -14" BELOW FINISHED FLOOR AT RWL. FOR LOCATIONS OF RWL SEE MECHANICAL DRAWINGS.

MASONRY

1. All available bearing areas of masonry units shall be fully covered with mortar, spreaded in an even layer and vertical joints shall be filled solidly with mortar.
2. All intersecting masonry walls to have masonry bond or heavy duty (block-lock) or equivalent at 8" vertically maximum.
3. For bonding brick and block use heavy duty truss type reinforcing or equivalent @ 8" vertically, maximum completely embedded in mortar.
4. Masonry walls shall be adequately braced to resist wind pressure.
5. All solid masonry shall be laid with full head and bed joints.
6. Provide all enclosures, heating and undertake methods of laying masonry in cold weather, in accordance with CSA Standard A224.
7. Provide lintels over all openings and recesses for mechanical and electrical trades as specified on Plans. See Architectural and Mechanical drawings for locations and sizes of openings and recesses.
8. Concrete and steel beams bearing on masonry walls shall have a minimum bearing of 8" unless otherwise noted on Plan.
9. Provide 3 courses of solid brick masonry under all bearing plates bearing on masonry for a distance of not less than 8" past bearing plate on each side.
10. Mortar shall be type "S" with a minimum compressive strength of a 9.5 MPa based on a net cross-sectional area.
11. Solid concrete block masonry with mortar type "M" or "S" shall have a minimum ultimate compressive strength of 7.5 MPa - f'm.
12. Hollow concrete block masonry with mortar type "M" or "S" shall have a minimum ultimate compressive strength of 9.8 MPa - f'm.
13. Compressive strength of concrete blocks shall be 15 MPa minimum, based on net cross-sectional area.
14. MASONRY DESIGN & CONSTRUCTION SHOULD COMPLY WITH CSA S304-14 (R2019) & CSA A370-14.

STEEL DECK

1. All fabrication and design confirm to CSA S136-01 and to CSB B1 Standard Specification
2. Allow for reinforcing at all openings up to 12" diameter as per Architect's & Mechanical Drawings.
3. Weld steel deck to joists, beams with 3/4" diameter fusion welds for diaphragm action at 12" o/c maximum. Side joints shall be mechanically clinched together @ 24" maximum.
4. All roof decks to be 1 1/2" x22GA. L.Z.C. deck, continuous over 3 supports minimum unless noted otherwise on plan.
5. Minimum thickness to be 0.030" (22GA) unless noted otherwise on drawings.
6. Deck connections to be as follows (U/N):
   - -Transverse Weld Spacing: 12" O/C
   - -Side Lap Button Punching: 24" O/C
   - -Longitudinal Weld Spacing: 36" O/C

CONCRETE AND REINFORCEMENT

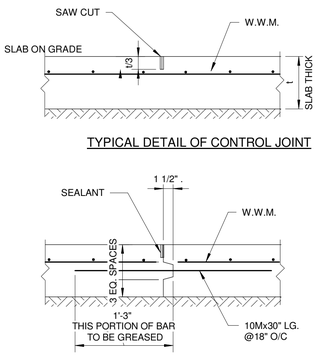
1. Concrete strength shall in no case be less than 25 MPa (U/N) after 28 days and concrete shall conform to CSA Specifications CAN/CSA-A23.3-19.
2. All reinforcing steel to be deformed bars to conform to CSA G30.12-M1977 with minimum fy=400 MPa unless otherwise noted on plan. Stirrups and ties to be deformed bars to conform to CSA G30.12 with minimum fy=350 MPa.
3. Concrete contractor to set all loose members that are to be embedded in the concrete. See Structural, Architectural and Mechanical Drawings.
4. Formwork contractor to form all holes, chases etc. and to set inserts, anchor bolts and other embedded members which are required to be held in place by the formwork before pouring the concrete. See Structural, Architectural and Mechanical Drawings.
5. Reinforcing bars in footings and slabs on earth and concrete members exposed for architectural reasons to weather, shall be supported in designated position by means of precast concrete supports or equivalent.
6. All poured concrete to be vibrated thoroughly.

BACKFILLING

1. BACKFILLING TO BE PROVIDED AS PER SOIL CONSULTANT RECOMENDATIONS.
2. AT GRADE WALL CONDITION BACKFILL EACH SIDE AF WALL SIMULTANEOUSLY.
3. ALL VERTICAL ELEMENTS SHOULD BE BRACED BY SLAB ON GRADE & CONCRETE SLAB ABOVE OR TEMPORARY SHORING BEFORE BACKFILLING.

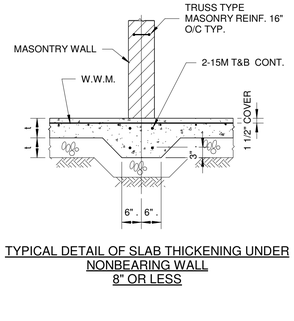
METAL STUD NOTES

1. STUD SIZES SHALL BE AS SHOWN ON DRAWINGS OR TO BE DESIGNED BY STUD SUPPLIER.
2. ERECTION, BRIDGING AND SHEATHING SHALL CONFORM TO MANUFACTURER'S SPECIFICATIONS.
3. SUBMIT SHOP DRAWINGS PREPARED & STAMPED BY A REGISTERED PROFESSIONAL ENGINEER FOR REVIEW AND APPROVAL.
4. ALL STUD TO STUD OR STUD TO STEEL CONNECTIONS TO BE DESIGNED BY STUD SUPPLIER.
5. STUDS INSTALLATION TO BE INSPECTED BY STUD SUPPLIER. CERTIFICATION LETTER FOR STUD INSTALLATION TO BE SUBMITTED STAMPED BY P. ENG.

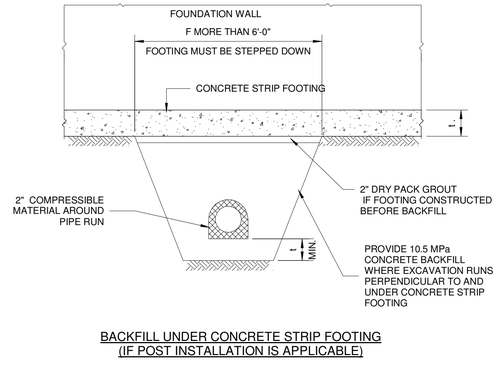


TYPICAL DETAIL OF CONTROL JOINT

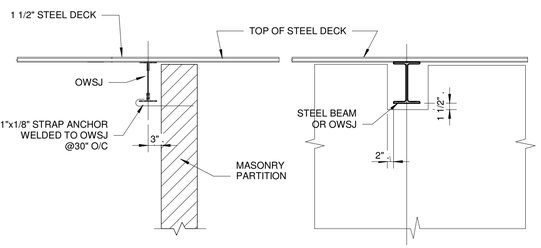
TYPICAL DETAIL OF CONSTRUCTION JOINT



TYPICAL DETAIL OF SLAB THICKENING UNDER NONBEARING WALL 8" OR LESS

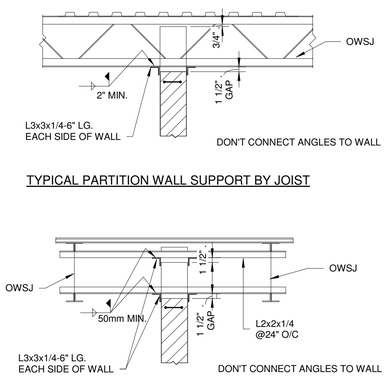


BACKFILL UNDER CONCRETE STRIP FOOTING (IF POST INSTALLATION IS APPLICABLE)



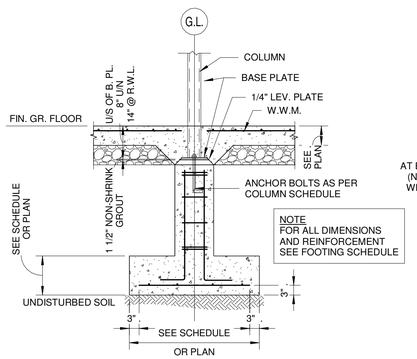
PARTITION LATERAL SUPPORT @ OWSJ

MASONRY PARTITION POCKETS @ OWSJ OR BEAMS

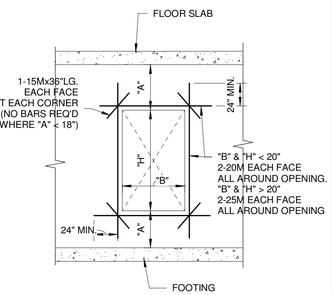


TYPICAL PARTITION WALL SUPPORT BY JOIST

TYPICAL PARTITION WALL SUPPORT BETWEEN JOISTS

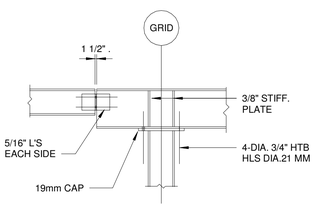


TYPICAL CONCRETE FOOTING CONSTRUCTION

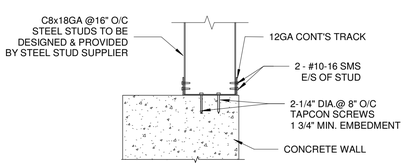


FOR LOCATION COORDINATE WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DWG`S.

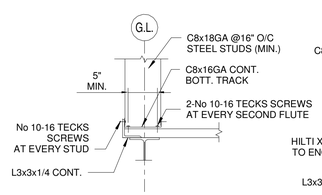
TYPICAL DETAIL OF OPENING IN WALL



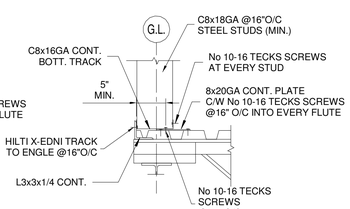
TYPICAL CANTILEVERED BEAM



TYPICAL CANTILEVERED STUD CONNECTION DETAIL FOR H EQUAL OR LESS THAN 3'-0"



TYPICAL DETAIL FOR PARAPET H < 2'-0"



TYPICAL DETAIL FOR PARAPET H <2'-0"

LINTELS IN NON-LOADBEARING BLOCK WALLS (ALL LINTELS SHOWN ARE L.L.V. U/N)

WALL THICK. mm / CL.OPENING	90	190	240	290
4'-0" OR LESS	1L3 1/2x3 1/2x1/4	2L3 1/2x3 1/2x1/4	1L3 1/2x3 1/2x1/4 1L5x3 1/2x1/4 (S.L.V)	3L3 1/2x3 1/2x1/4
5'-0"	1L3 1/2x4x1/4	2L3 1/2x4x1/4	1L3 1/2x4x1/4 1L5x3 1/2x1/4 (S.L.V)	3L3 1/2x3 1/2x1/4
6'-0"	1L3 1/2x5x1/4	2L3 1/2x5x1/4	1L3 1/2x5x1/4 1L5x5x5/16	3L3 1/2x5x1/4
7'-0"	1L3 1/2x5x3/8	2L3 1/2x5x3/8	1L3 1/2x5x5/16 1L5x5x5/16	3L3 1/2x5x5/16

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16/06/2023
PROVINCE OF ONTARIO

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DRAWING TITLE
GENERAL NOTES AND TYPICAL DETAILS

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G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	As indicated
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-01.1-C5**

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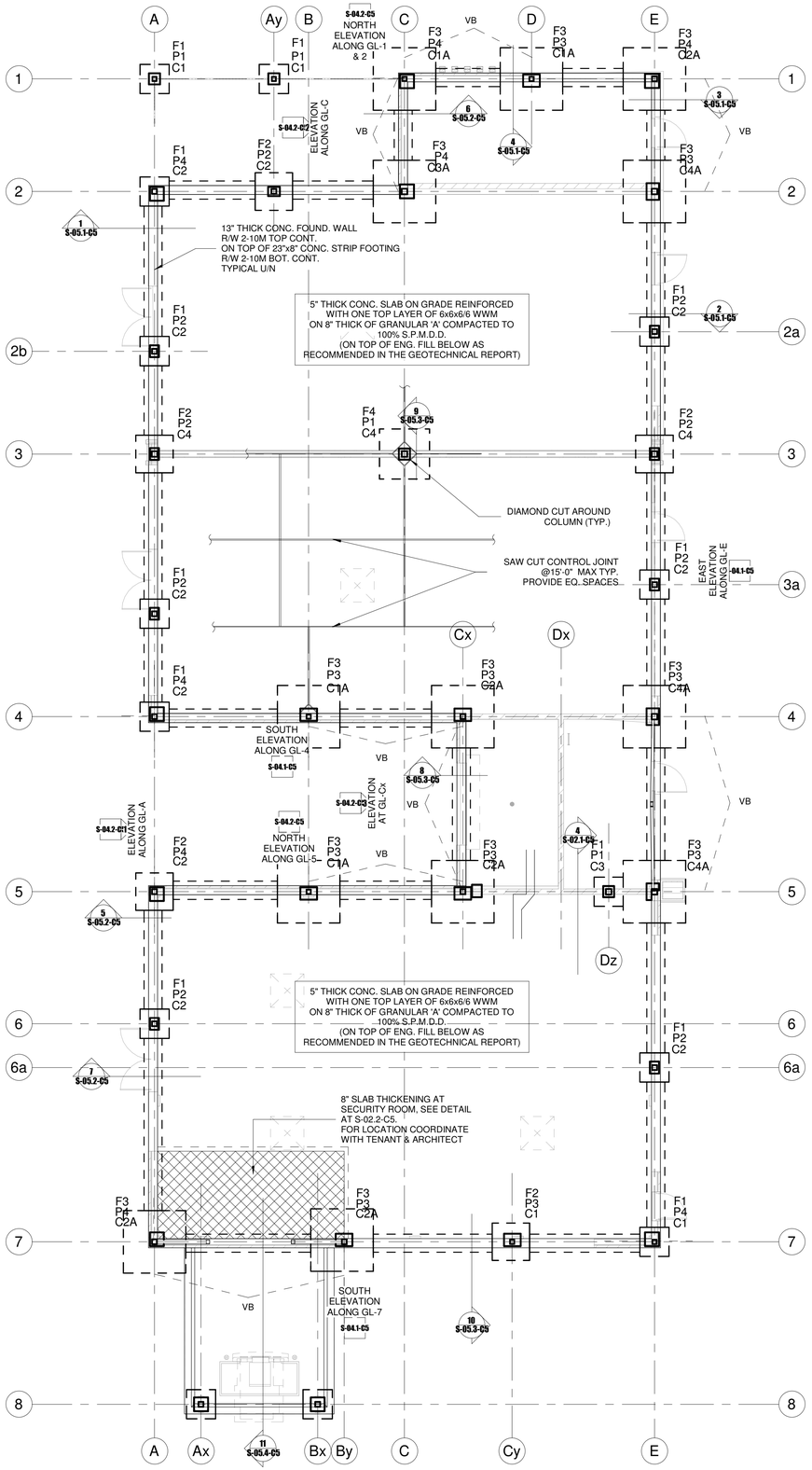
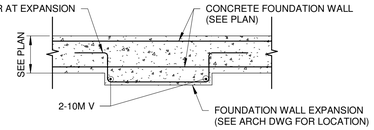


1 FOUNDATION PLAN
S-02.1-C5 1/8" = 1'-0"

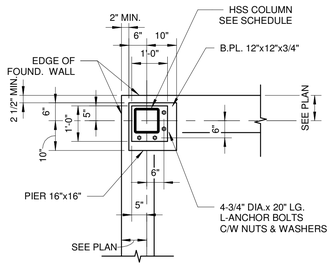
PIER SCHEDULE:

DATA	MARK	P1	P2	P3	P4
CONCRETE PIERS	SIZE	16" x 16"	13" x 17" LONG SIZE ALONG FOUNDATION WALL	18" x 24" LONG SIZE ALONG FOUNDATION WALL	20" x 20" CORNER
REINF.	VERT.	4-15M	4-15M	6-15M + 3-15M TOP U-BARS (24" MIN. EMBEDMENT)	6-15M + 3-15M TOP U-BARS (24" MIN. EMBEDMENT)
	TIES	10M @ 12"O/C			
	SHAPE			U-BARS	U-BARS

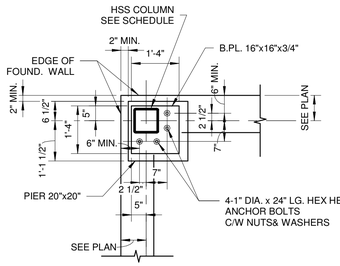
NOTE : FOR BASE PLATES AND ANCHOR BOLTS (SEE COLUMN SCHEDULE ON DRAWING S-03.1-A5)



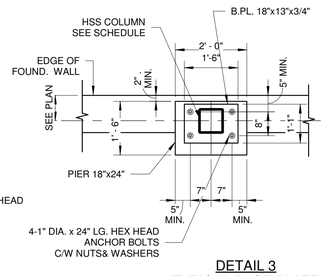
A FOUNDATION WALL BUMP-OUT DETAIL
S-02.1-C5 1/2" = 1'-0"



DETAIL 1
CORNER BASE PLATE U/N



DETAIL 2
CORNER BASE PLATE
AT COLUMNS WITH INDEX "A"



DETAIL 3
TYPICAL BASE PLATE
FOR EXTERIOR COLUMNS WITH INDEX 'A'

CONVENTIONAL FOOTING SCHEDULE:

DATA	MARK	F1	F2	F3	F4
CONCRETE FOOTINGS	SIZE	3'-6" x 3'-6"	4'-6" x 4'-6"	7'-6" x 7'-6"	6'-0" x 6'-0"
	THICKNESS	12"	12"	24"	16"
	RW BOT.	4-15M E/W	5-15M E/W	9-20M E/W	8-15M E/W
	RW TOP	N/A	N/A	9-20M E/W	N/A

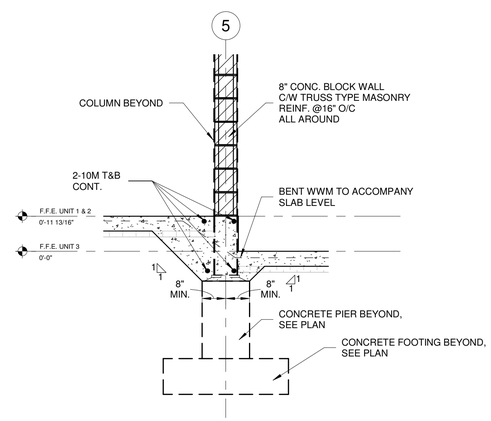
NOTE :
FOOTINGS FOR BRACED COLUMNS MUST HAVE TOP REINFORCEMENT TO MATCH BOTTOM.

FOUNDATION PLAN NOTES:
PN-FND

1. SEE SOIL INVESTIGATION REPORT PREPARED BY "GOLDER ASSOCIATES LTD." REFERENCE #20146060(1000), DATED OCTOBER 19, 2020, AND TECHNICAL ADDENDUM DATED DECEMBER 16,2022.
2. FOOTING SHALL BE CARRIED DOWN TO UNDISTURBED NATIVE SOIL OR ENGINEERED FILL WITH A GEOTECHNICAL RESISTANCE AT U.L.S. OF 350 kPa. THE GEOTECHNICAL REACTION AT S.L.S. OF 250 kPa. (SEE SOIL CONSULTANT RECOMMENDATIONS).
3. SOIL AT THE UNDERSIDE OF THE FOOTING IS TO BE INSPECTED AND APPROVED BY SOIL CONSULTANT PRIOR TO PLACING CONCRETE.
4. ALL REINFORCING STEEL TO BE DEFORMED BARS TO CONFORM TO CSA G30.18-M1992 WITH MIN. Fy=400 MPa U/N. STIRRUPS AND TIES TO BE DEFORMED BARS TO CONFORM TO CSA G30.18 WITH MIN. Fy=350 MPa.
5. ALL FOOTINGS (INTERIOR, PERIMETER & EXTERIOR) TO BE A MIN. OF 1.4m (APPROX. 4'-8") BELOW FINISHED GRADE. SEE GEOTECHNICAL REPORT FOR ANTICIPATED BEARING DEPTHS
6. THE LINE OF SLOPE BETWEEN ADJACENT FOOTING IS TO BE A MAX. OF 7 IN 10, STEPS TO BE 2'-0" MAX.
7. ALL SLAB ON GRADE SHOULD BE CONSTRUCTED ON COMPACTED GRANULAR MATERIAL AND TO BE APPROVED BY THE GEOTECHNICAL CONSULTANT.
8. CONCRETE SLAB ON GRADE SEE PLAN FOR DETAILS.
9. CONCRETE COMPRESSIVE STRENGTH FOR FOOTINGS TO BE 20 MPa AT 28 DAYS.
10. CONCRETE COMPRESSIVE STRENGTH FOR FOUNDATION WALLS TO BE 25 MPa AT 28 DAYS. CONCRETE EXPOSURE CLASS F-2.
11. CONCRETE COMPRESSIVE STRENGTH FOR ALL SLAB ON GRADE TO BE 25 MPa AT 28 DAYS.
12. CONCRETE COMPRESSIVE STRENGTH FOR ALL EXTERIOR SLAB ON GRADE TO BE 35 MPa AT 28 DAYS. CONCRETE EXPOSURE CLASS C-1.
13. ALL CONCRETE WORK SHALL CONFORM TO CAN/CSA-A23.1 AND CAN/CSA-A23.2 LATEST EDITIONS
14. THIS DRAWING TO BE READ IN CONJUNCTION WITH THE RELEVANT ARCHITECTURAL DRAWINGS.
15. CONCRETE CONTRACTOR TO SET ALL LOOSE MEMBERS THAT ARE TO BE EMBEDDED IN THE CONCRETE. SEE STRUCTURAL, ARCHITECTURAL AND MECHANICAL DRAWINGS.
16. FORM WORK CONTRACTOR TO FORM ALL HOLES, CHASES etc. AND TO SET INSERTS, ANCHOR BOLTS AND OTHER EMBEDDED MEMBERS ARE REQUIRED TO BE HELD IN PLACE BY THE FORM WORK BEFORE POURING THE CONCRETE. SEE STRUCTURAL, ARCHITECTURAL AND MECHANICAL DRAWINGS FOR LOCATION.
17. REINFORCING BARS IN FOOTINGS AND SLABS ON EARTH AND CONCRETE MEMBERS EXPOSED FOR ARCHITECTURAL REASONS TO WEATHER, SHALL BE SUPPORTED IN DESIGNATED POSITION BY MEANS OF PRECAST CONCRETE SUPPORTS OR EQUIPMENT.
18. ALL POURED CONCRETE TO BE VIBRATED THOROUGHLY.
19. PROVIDE CONTROL JOINT AT OUTSIDE FACE OF FOUNDATION WALL AT 30'-0" (9 m) O/C (TYPICAL U/N).
20. CENTER LINE OF COLUMN SHOULD BE AT CENTER OF FOOTING (TYPICAL U/N).
21. SEE CIVIL AND ARCH. DWGS FOR FROST SLAB LOCATIONS.

BACKFILL

1. SLABS ON GRADE ON ALL STRUCTURAL ELEMENTS FRAMED INTO THE WALLS, WHICH ARE RETAINING EARTH, MUST BE IN PLACE BEFORE BACKFILLING.
2. AT GRADE WALL CONDITIONS BACKFILL EACH SIDE OF WALL SIMULTANEOUSLY.



4 STEPPED SLAB DETAIL
S-02.1-C5 1/2" = 1'-0"

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NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY
		REVISION	

LICENSED PROFESSIONAL ENGINEER
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16/06/2023
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PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
FOUNDATION PLAN

DRAWN BY	DATE
G.R.	2023/03/13
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S.Sv.	As indicated
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22-3849	**S-02.1-C5**

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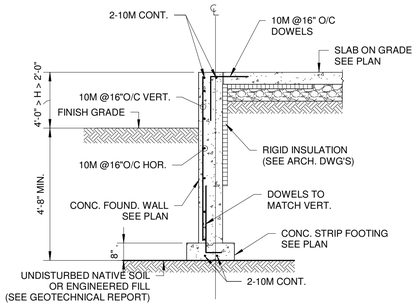
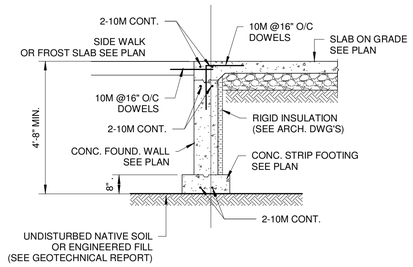
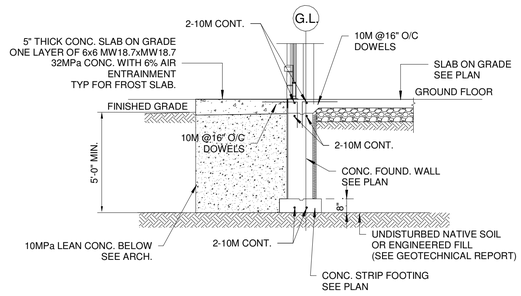


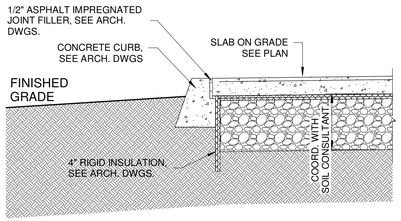
TYPICAL SECTION FOR LOW GRADE WALL
WHERE H>2'-0"



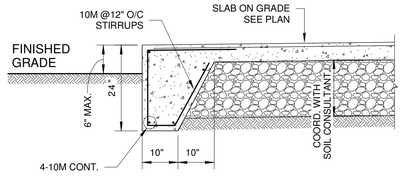
TYPICAL SECTION AT DOOR LOCATION
FOR CONVENTIONAL FOOTING



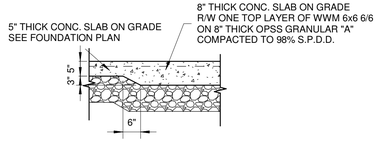
TYPICAL FROST SLAB SECTION
WITH CONVENTIONAL FOOTINGS



TYPICAL DETAIL "C"
EXTERIOR CONCRETE FROST SLAB EDGE DETAIL



TYPICAL DETAIL "B"
EXTERIOR CONCRETE SLAB EDGE DETAIL



TYPICAL SLAB THICKENING DETAIL

N

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1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY

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BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
FOUNDATION TYPICAL DETAILS

DRAWN BY	DATE
G.R.	2023/03/13
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22-3849	**S-02.2-C5**

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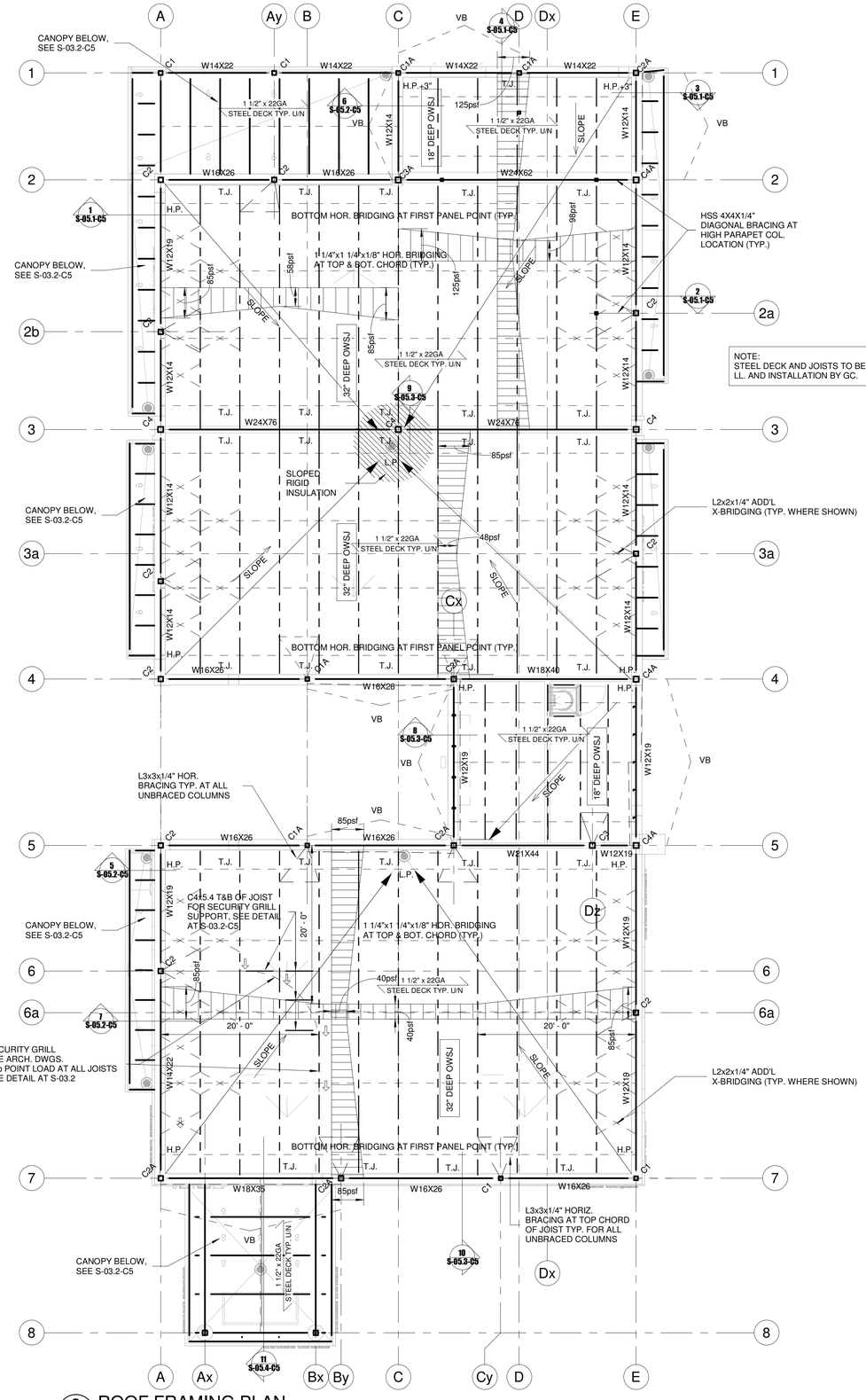


3 ROOF FRAMING PLAN
S-03.1-C5 1/8" = 1'-0"

COLUMN SCHEDULE				
DATA / MARK	C1	C2	C3	C4
SIZE SHAPE	HSS6x6x0.25	HSS7x7x0.25	HSS8x8x0.25	HSS8x8x0.375
BASE PLATE	14"x3/4"x11"	14"x3/4"x11"	14"x3/4"x14"	12"x3/4"x12"
ANCHOR BOLTS	4 - 3/4" DIA. x 20" LG HEX HEAD	4-Ø1"x24"LG HEX HEAD	6-Ø1"x24"LG HEAVY HEX HEAD W/ 3"x3"x1/2" PLATE	4 - 3/4" DIA. x 20" LG HEX HEAD

NOTES:

1. USE STEEL Fy=350 MPa HSS CLASS C.
2. FOR CORNER COLUMN USE CORNER BASE PLATE SEE DETAILS '1'
3. COLUMN WITH INDEX "A" TO HAVE 4-Ø1"x24"LG HEX HEAD ANCHOR BOLTS. SEE DETAILS '2' , '3'.

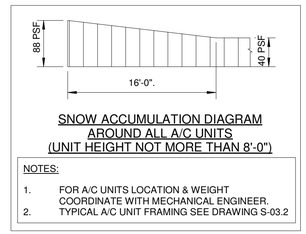
NOTE:
STEEL DECK AND JOISTS TO BE SUPPLIED BY LL. AND INSTALLATION BY GC.

ROOF FRAMING PLAN NOTES: PN-RF

1. FOR TOP OF STEEL ELEVATION & SLOPES SEE ARCHITECTURAL DRAWINGS.
2. ROOF LOADS:
   DL= 15 psf
   SL= 40 psf + SNOW ACCUMULATION
   LL= 20 psf & 0.3 kip POINT LOAD AT ANY LOCATION
   LOAD COMBINATIONS FOR ULS: 1.25xDL+1.5xSL
   LOAD COMBINATIONS FOR SLS: 0.9xSL
   MAXIMUM SL OR LL DEFLECTION = L/300 TYPICAL U/N.
   ALL LOADS SHOWN ON PLAN ARE UNFACTORED LOADS U/N.
3. FOR ALL MECHANICAL EQUIPMENT ALLOW AN ADDITIONAL POINT LOAD ON JOIST ANYWHERE ALONG THE JOIST SPAN.
4. FABRICATION AND ERECTION OF STRUCTURAL STEEL TO CONFORM TO THE STANDARD CAN/CSA S16-01 "LIMIT STATE DESIGN OF STEEL STRUCTURES".
5. WELDING SHALL CONFORM TO STANDARDS W47.1, W48.1 AND W59.1 LATEST EDITION.
6. ALL EXPOSED STEEL TO BE HOT DIP GALVANIZED.
7. ALL STRUCTURAL STEEL TO BE 350W AND CONFORM TO CAN/CSA G40.21.
8. BRIDGING, BRACING AND ANCHORAGE TO CONFORM TO CAN/CSA S16-01, OBC LATEST EDITION.
9. PROVIDE 11/2"x22 GA L.Z.C. STEEL DECK (TYPICAL) U/N. SEE STRUCTURAL DRAWING S-01 FOR CONNECTION.
10. ALL JOISTS TO HAVE 4" DEEP SHOES U/N. ALL JOISTS TO BE WELDED TO SUPPORTING STEEL. SIZE AND SPACING AS SHOWN ON PLAN.
11. ALL JOISTS MARKED "T.J." ARE THE JOIST TO HAVE DOUBLE ANGLE AT TOP AND BOTTOM CHORD, AND THE BOTTOM CHORD TO BE EXTENDED AND CONNECTED TO COLUMNS OR BEAMS.
12. PROVIDE TIE JOIST AS SHOWN ON PLAN.
13. ALL JOISTS AND DECK SHOULD BE DESIGNED BY SUPPLIER TO RESIST UPLIFT PRESSURE DUE TO WIND IN ACCORDANCE WITH NBC SUPPLEMENT C#4. FOR UPLIFT LOADS SEE NET ROOF WIND UPLIFT DIAGRAM.
14. FRAME ALL OPENINGS IN THE ROOF WITH C4X5.4 TYPICAL UNLESS OTHERWISE NOTED. EXACT LOCATION COORDINATE WITH THE ARCHITECT & THE MECHANICAL CONSULTANT.
15. H.P. AND L.P. DENOTES HIGH AND LOW POINTS.
16. ALL JOISTS SUPPORTING SPRINKLER LINES EQUAL OR GREATER THAN 4" DIAMETER SHOULD BE DESIGNED FOR ADDITIONAL LOAD OF 0.4 kip/ft ALONG THE JOIST. COORDINATE WITH FIRE CONSULTANT FOR LOCATION. ALL SPRINKLER LINE CONNECTIONS TO BE AT JOIST TOP CHORD ONLY (AT PANEL POINT).
17. PROVIDE L3x3x1/4" WELDED ON TOP OF JOIST OR BEAM CONTINUOUS ALL AROUND ROOF PERIMETER. PERIMETER ANGLE TO BE WELDED TO COLUMN OR POST/STUB WHERE INTERRUPTED - SPLICE DETAIL AT COLUMNS TO BE DESIGNED BY STEEL SUPPLIER AND SUBMITTED FOR STRUCTURAL REVIEW AND RECORD.
18. DESIGN JOIST FOR ADDITIONAL LOAD OF 25 psf AT CONCRETE PAVERS ON TOP OF THE ROOF. FOR LOCATION COORDINATE WITH ARCHITECT.
19. ALL JOISTS SUPPORTING SPRINKLER LINES GREATER THAN 4" DIAMETER SHOULD BE DESIGNED FOR ADDITIONAL LOAD OF 40plf ALONG THE JOIST. COORDINATE WITH FIRE CONSULTANT FOR LOCATION. ALL SPRINKLER LINE CONNECTIONS TO BE AT JOIST TOP CHORD ONLY (AT PANEL POINT).
20. CONNECTION DETAILS FOR ALL MECHANICAL EQUIPMENT SUCH AS PIPES, DUCTS, ROOF TOP UNITS ETC. TO BE DESIGNED BY MECHANICAL ENGINEER FOR SEISMIC FORCES IN ACCORDANCE WITH DIVISION B, ARTICLE 4.1.8.17 OF OBC 2012.
21. SEE ALSO TYPICAL DETAILS AND NOTES ON DWG. S-01.

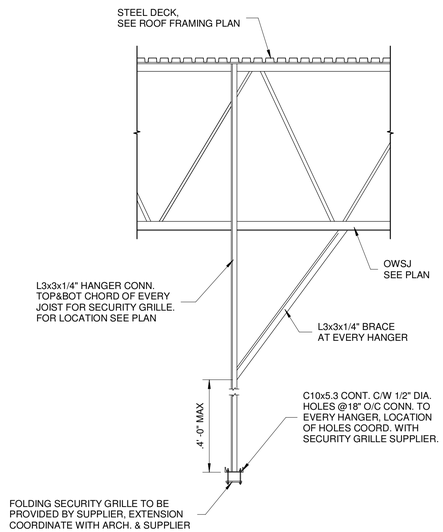
METAL STUD NOTES:

1. GAUGE & SPACING OF STUDS ON DRAWINGS IS THE MINIMUM ACCEPTABLE SIZE FOR SHEATHING SUPPORT. STUD GAUGE AND SPACING TO BE DESIGNED BY THE STUD SUPPLIER.
2. ERECTION, BRIDGING AND SHEATHING SHALL CONFIRM TO MANUFACTURER'S SPECIFICATIONS.
3. SUBMIT SHOP DRAWINGS PREPARED AND STAMPED BY A REGISTERED PROFESSIONAL ENGINEER FOR REVIEW AND APPROVAL.
4. CONNECTION OF STUDS TO STEEL TO BE DESIGNED BY STUD SUPPLIER.
5. STUDS INSTALLATION TO BE INSPECTED BY STUD SUPPLIER.CERTIFICATION LETTER FOR STUD INSTALLATION TO BE SUBMITTED AND STAMPED BY P.ENG.

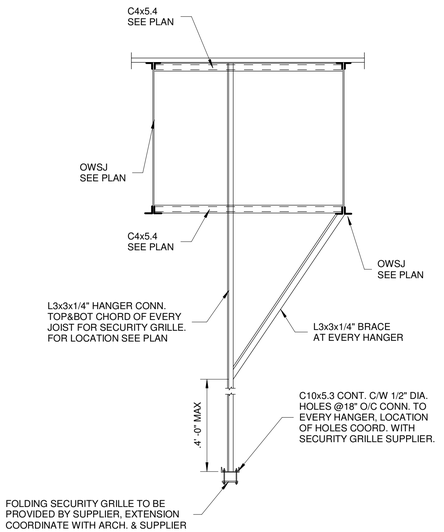


SNOW ACCUMULATION DIAGRAM AROUND ALL A/C UNITS (UNIT HEIGHT NOT MORE THAN 8'-0")

NOTES:
1. FOR A/C UNITS LOCATION & WEIGHT COORDINATE WITH MECHANICAL ENGINEER.
2. TYPICAL A/C UNIT FRAMING SEE DRAWING S-03.2



TYPICAL DETAIL FOR FOLDING SECURITY GRILLE SUPPORT AT JOIST LOCATION



TYPICAL DETAIL FOR FOLDING SECURITY GRILLE SUPPORT BETWEEN JOIST

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1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
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4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

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PROJECT
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BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
ROOF FRAMING PLAN

DRAWN BY	DATE
G.R.	2023/03/13
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S.Sv.	As indicated
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-03.1-C5**

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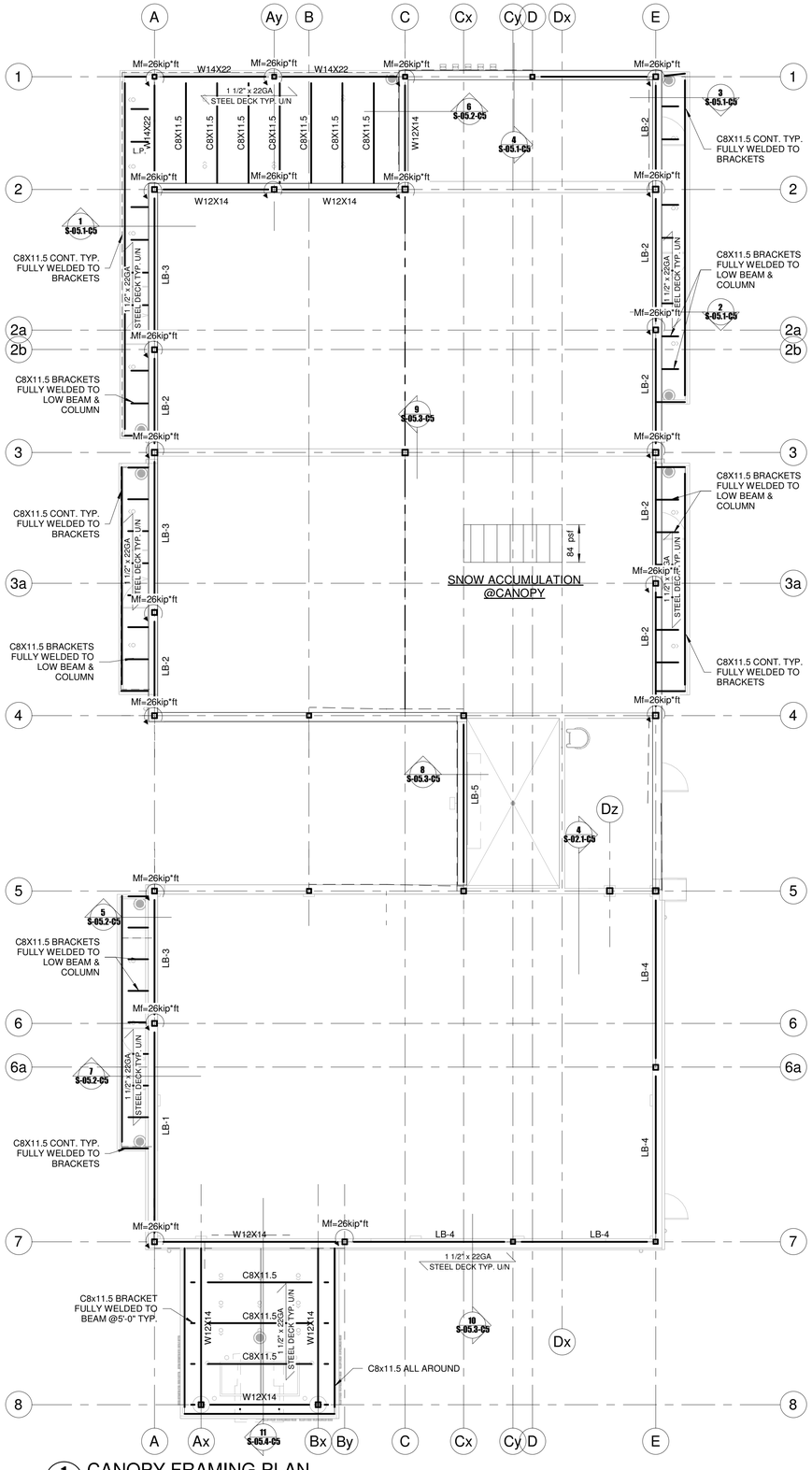
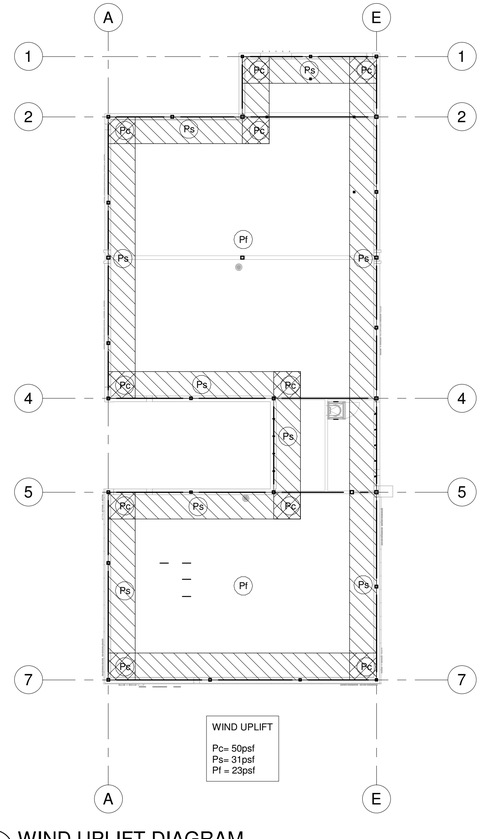
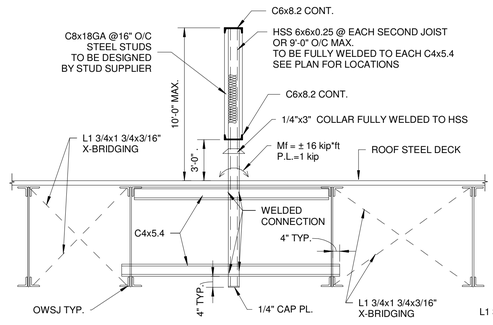


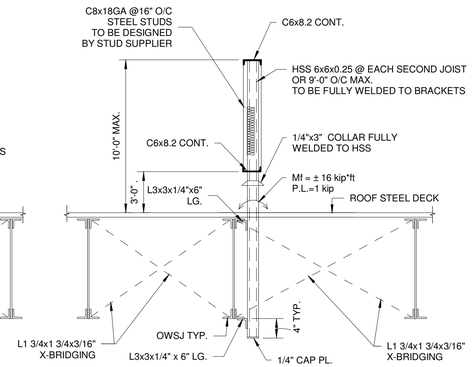
1 CANOPY FRAMING PLAN
S-03.2-C5 1/8" = 1'-0"



2 WIND UPLIFT DIAGRAM
S-03.2-C5 1/16" = 1'-0"



TYPICAL ROOF SCREEN POST CONNECTION DETAIL BETWEEN JOIST LOCATIONS U/N ON PLAN



TYPICAL ROOF SCREEN POST CONNECTION DETAIL AT JOIST LOCATION U/N ON PLAN

NOTES:

1. ALL EXPOSED STEEL TO BE HOT DIP GALVANIZED.
2. FOR LOCATION & EXTEND SCREEN WALL SEE ARCH. DWG'S

CANOPY FRAMING PLAN NOTES:

PN-CP

01. FOR TOP OF STEEL ELEVATION & SLOPES SEE ARCHITECTURAL DRAWINGS.

02. CANOPY ROOF LOADS:
DL= 15 PSF
SL= 84 PSF @ ALL CANOPY
LL= 20PSF & 5 KIP POINT LOAD AT ANY LOCATION WHICHEVER GOVERNS

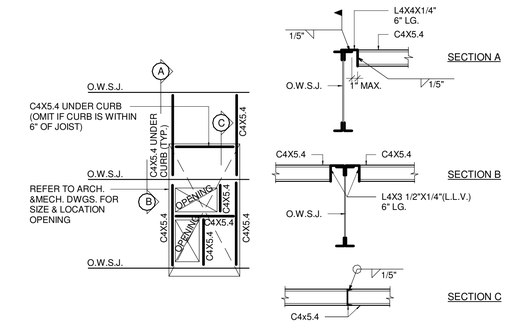
LOAD COMBINATIONS FOR ULS & SLS AS PER NBC REQUIREMENTS .

MAXIMUM SL OR LL DEFLECTION = L/240.

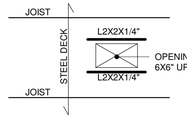
03. DESIGN ANTI-ROTATIONAL CONNECTION OF LOW BEAM TO COLUMN FOR THE OUT OF PLANE MOMENT Mf (kip* ft) TYPICAL WHERE SHOWN. Mf=26kip*ft

FOR VALUES SEE PLAN.

TYPICAL A/C UNIT & SMALL ROOF OPENING FRAMING



A/C UNIT FRAMING



NOTES:
1. PROVIDE L2X2X1/4" UNDER THE FLUTES AT EACH SIDE OF THE OPENING.
2. EXTEND ACROSS AT LEAST THREE FLUTES ON EACH SIDE OF THE OPENING.
3. EACH ANGLE TO BE ATTACHED TO EXIST. STEEL DECK W/ 1/4" DIA. SCREWS @6" O/C. MAX. (USE 3 SCREWS MN.)

(VERT. LOADS TO THE DECK AND ANGLES NOT PERMITTED)

SMALL ROOF OPENING FRAMING

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1	2023-03-16	FOR 50% COORDINATION	S.Sv.
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DRAWING TITLE
CANOPY FRAMING PLAN

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S.Sv.	As indicated
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22-3849	**S-03.2-C5**

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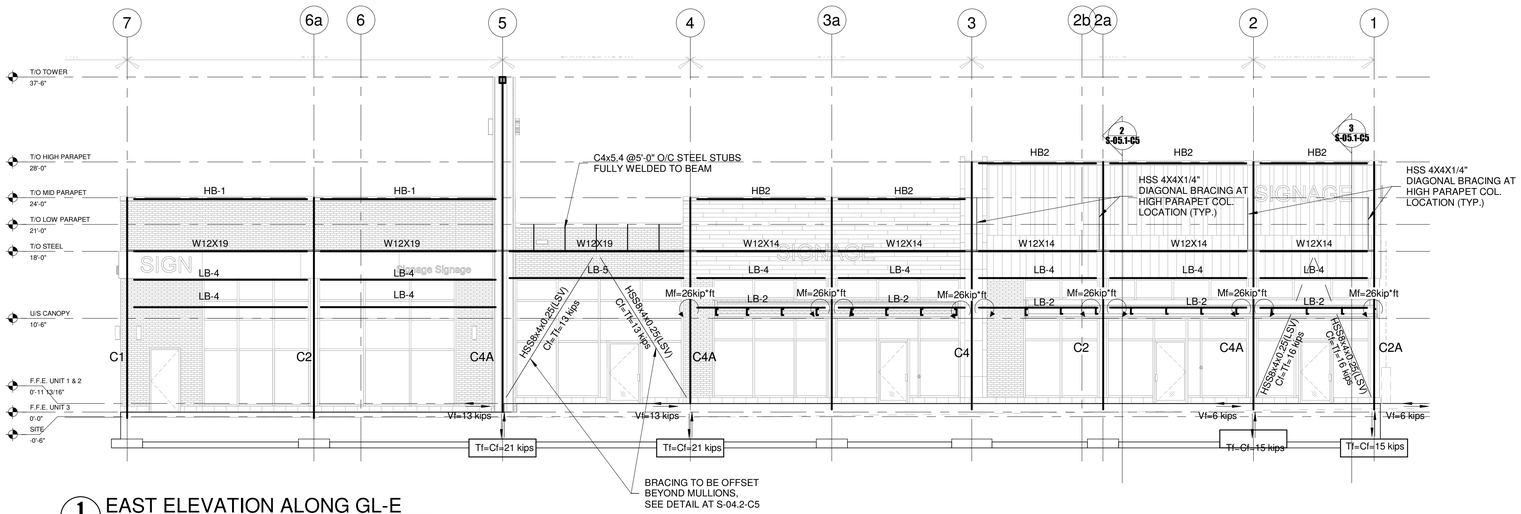
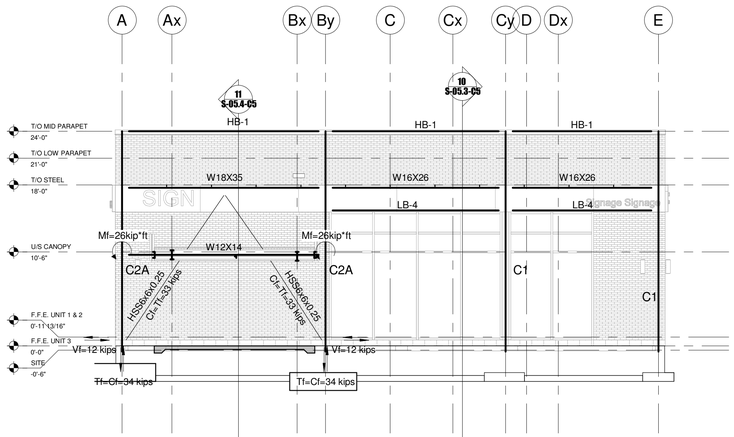
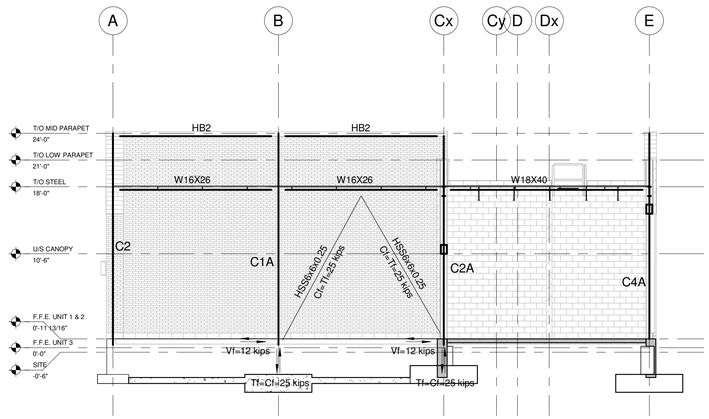


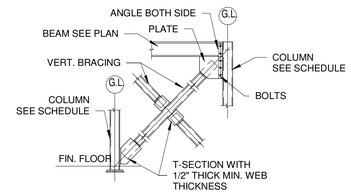
1 **S-04.1-C5** EAST ELEVATION ALONG GL-E
1/8" = 1'-0"



2 **S-04.1-C5** SOUTH ELEVATION ALONG GL-7
1/8" = 1'-0"



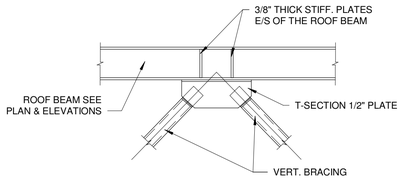
3 **S-04.1-C5** SOUTH ELEVATION ALONG GL-4
1/8" = 1'-0"



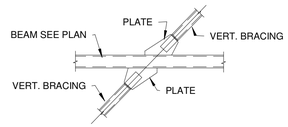
TYPICAL BRACING CONNECTION DETAIL

NOTES:

1. BRACE & BRACE CONNECTION DESIGN LOADS SEE ON THE ELEVATION DRAWINGS

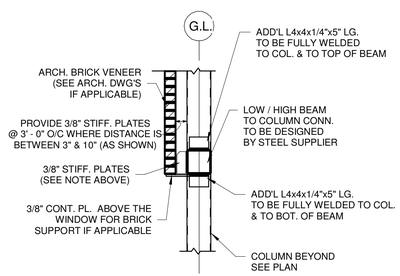


DETAIL "A"
BRACING CONNECTION

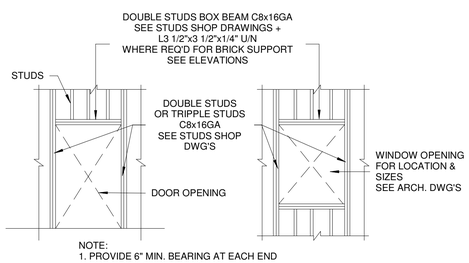


DETAIL "B"
BRACING CONNECTION

OFFSET BRACING CONNECTION DETAIL



TYPICAL LOW / HIGH BEAM CONNECTION DETAIL AT BRICK VENEER SUPPORT LOCATIONS



TYPICAL DOOR OPENING

TYPICAL WINDOW OPENING

LOW BEAM SCHEDULE

MARK / DATA	LB-1	LB-2	LB-3	LB-4	LB-5
SIZE SHAPE	HSS12x8x0.313 (L.S.V.)	HSS8x8x0.25	HSS12x8x0.25 (L.S.V.)	HSS8x4x0.25 (L.S.H.)	HSS12x8x0.25 (L.S.V.)

HIGH BEAM SCHEDULE

MARK / DATA	HB-1	HB-2	HB-3
SIZE SHAPE	HSS8x4x0.25 (LSH)	HSS8x4x0.188 (LSH)	HSS8x8x0.25

LOW/ HIGH BEAM SCHEDULE

NOTES:

1. USE STEEL Fy=350 MPa HSS CLASS C.
2. FOR LOCATION LOW & HIGH BEAMS SEE ELEVATIONS.
3. DESIGN ANTI-ROTATIONAL CONNECTION FOR ALL LOW BEAM TO COLUMN FOR THE OUT OF PLANE MOMENT Mf=26 (ft-kips) TYPICAL.

VERTICAL BRACES NOTES:

1. MAX. OUT OF PLANE = 5 kips E/E

2. ALL BRACING LOADS ARE MAX FACTORED WIND OR SEISMIC LOADS.
3. CENTER LINE OF VERTICAL BRACING TO BE AT CENTER LINE OF COLUMNS U/N.

N

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO DATE DESCRIPTION BY
REVISION

LICENSED PROFESSIONAL ENGINEER
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PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
BUILDING ELEVATIONS

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	As indicated
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-04.1-C5**

2023-07-14 1:00:08 PM

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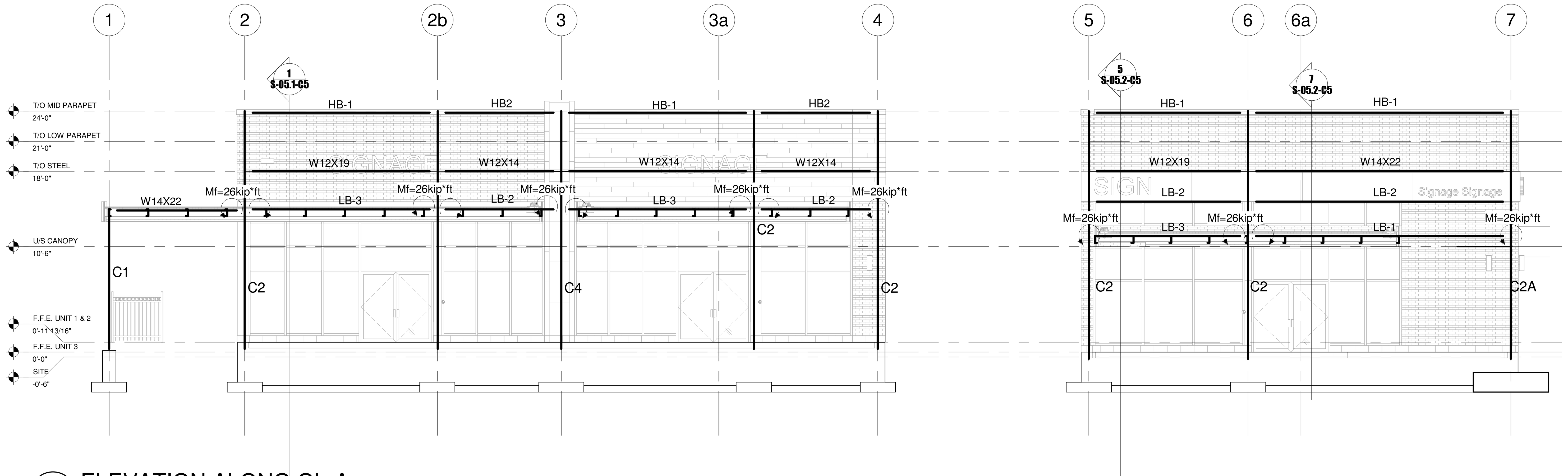
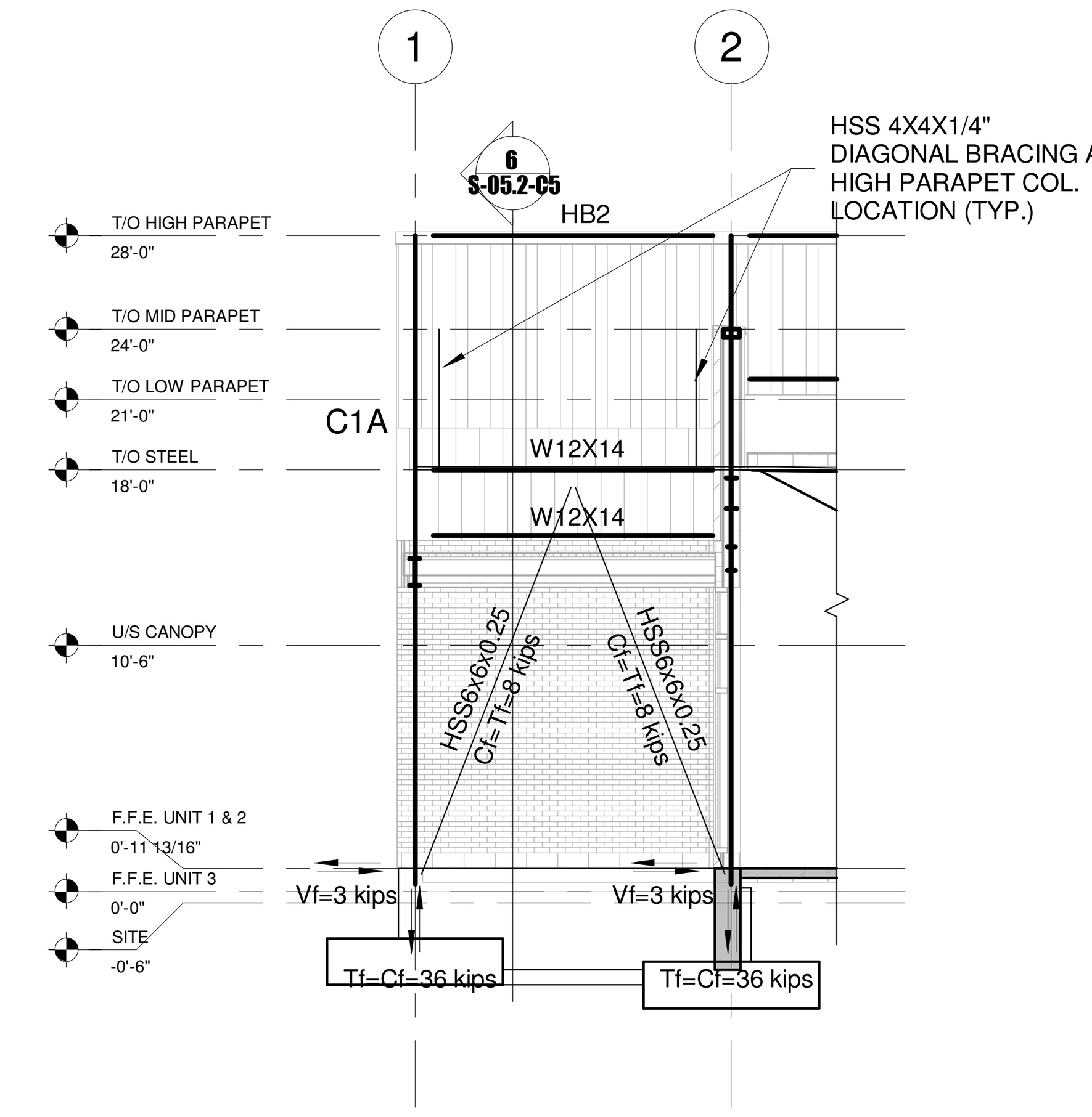
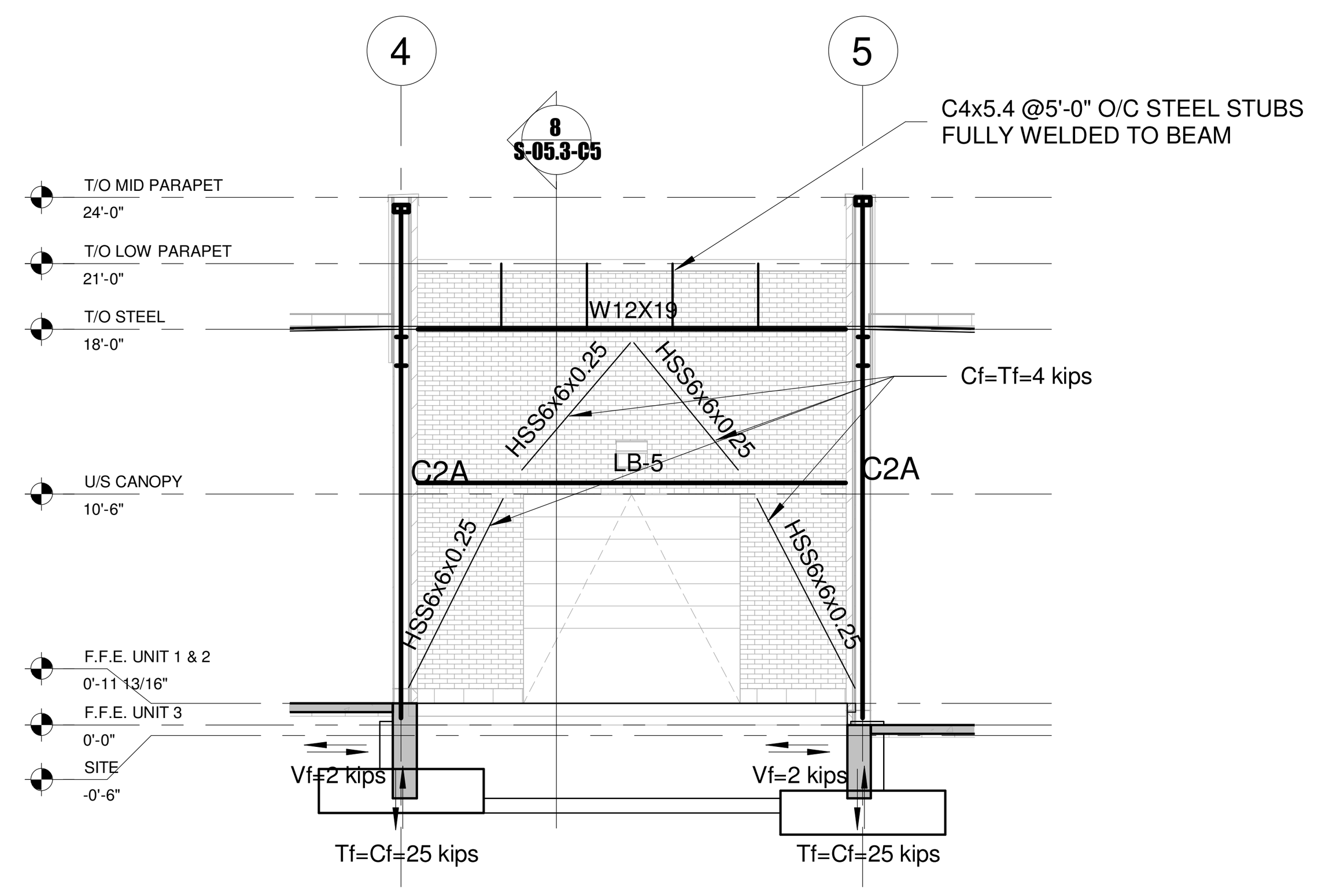


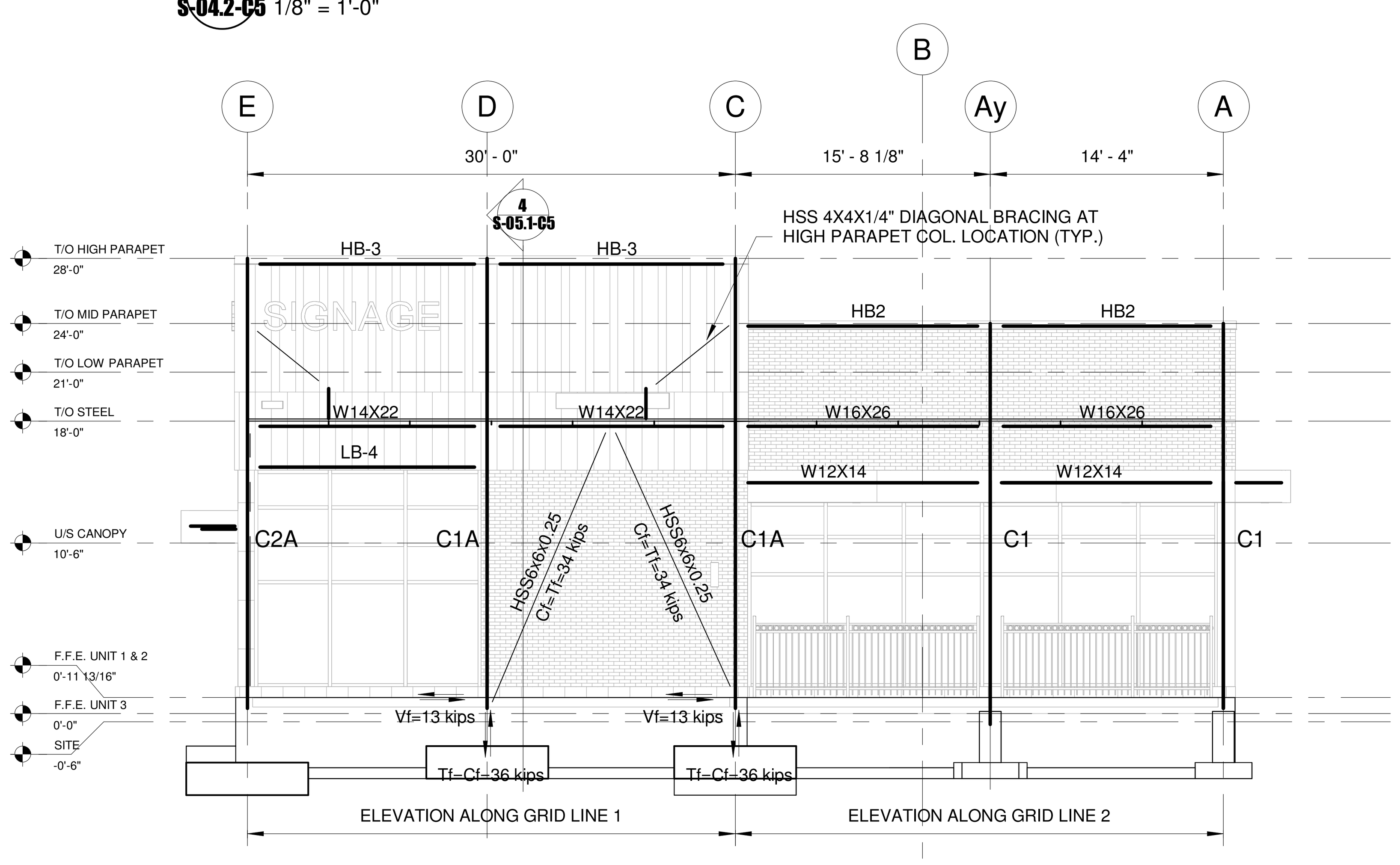
1 ELEVATION ALONG GL-A
S-04.2-C5 1/8" = 1'-0"



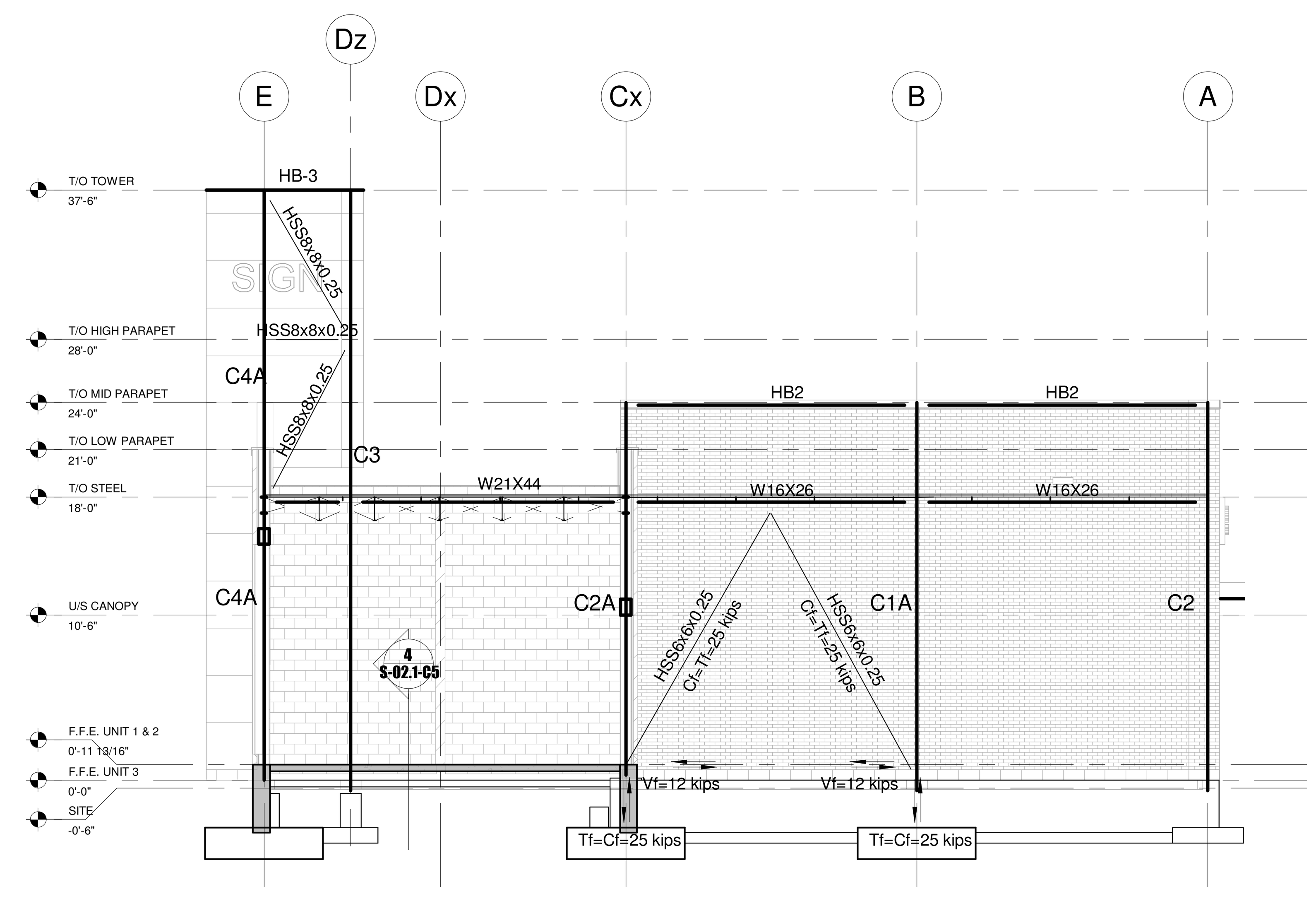
2 ELEVATION ALONG GL-C
S-04.2-C5 1/8" = 1'-0"



3 ELEVATION AT GL-Cx
S-04.2-C5 1/8" = 1'-0"



4 NORTH ELEVATION ALONG GL-1 & 2
S-04.2-C5 1/8" = 1'-0"



5 NORTH ELEVATION ALONG GL-5
S-04.2-C5 1/8" = 1'-0"

N

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY

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PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
BUILDING ELEVATIONS

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	1/8" = 1'-0"
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-04.2-C5**

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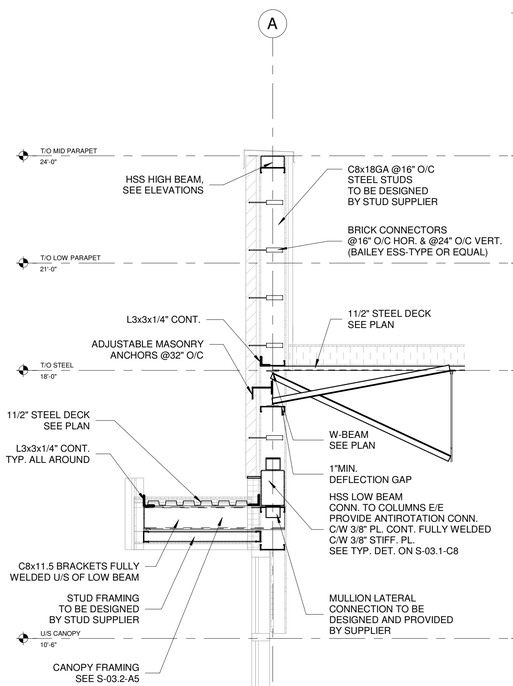
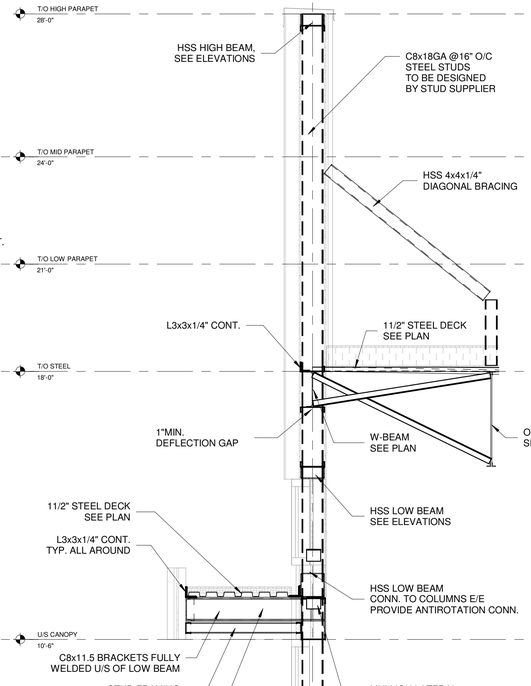


1 Section 1
S-05.1-C5 1/2" = 1'-0"



2 Section 2
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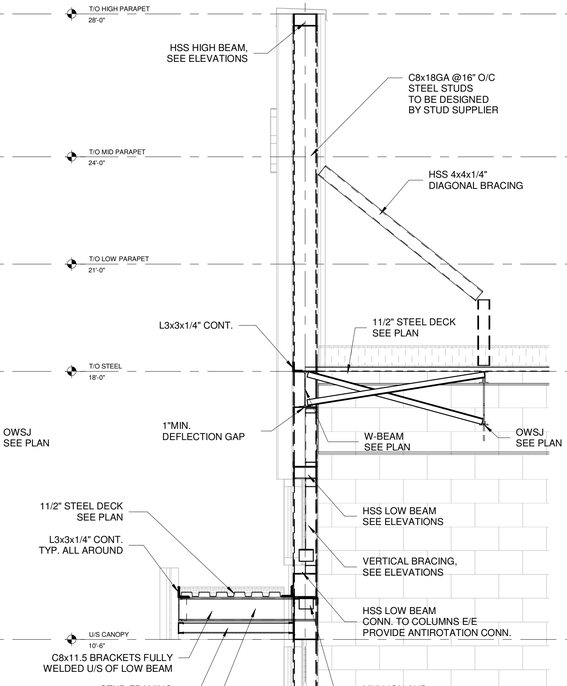
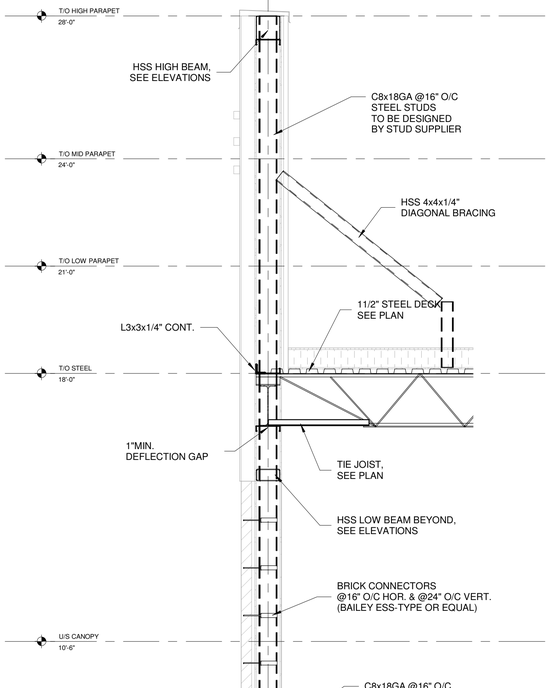


3 Section 3
S-05.1-C5 1/2" = 1'-0"



4 Section 4
S-05.1-C5 1/2" = 1'-0"

N

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY

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PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
SECTIONS

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	1/2" = 1'-0"
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-05.1-C5**

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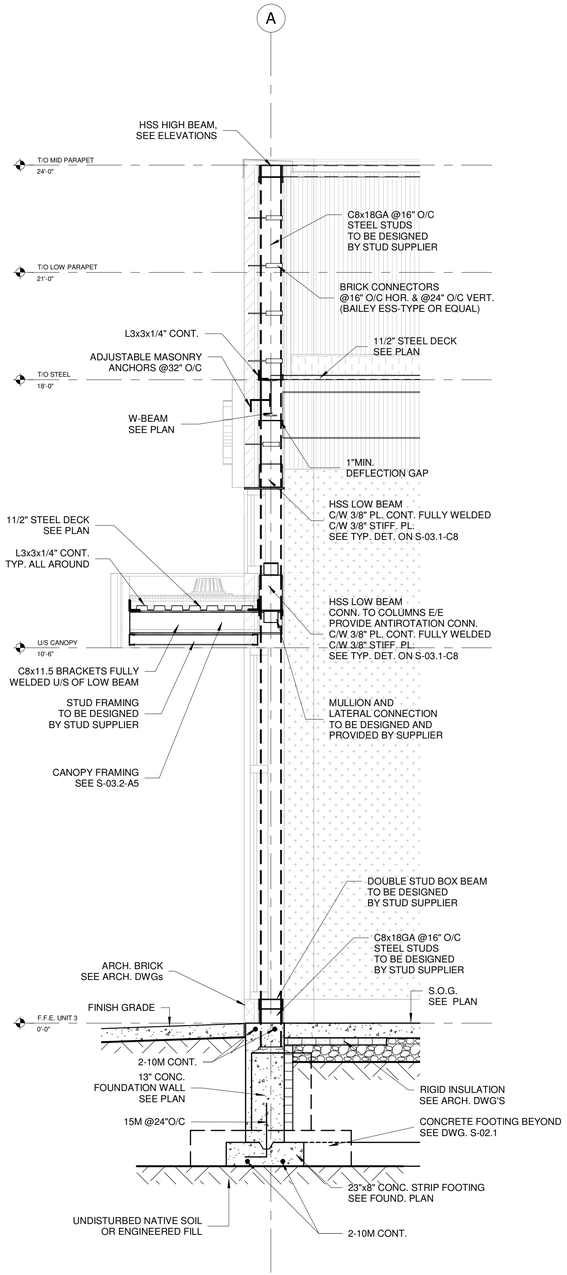
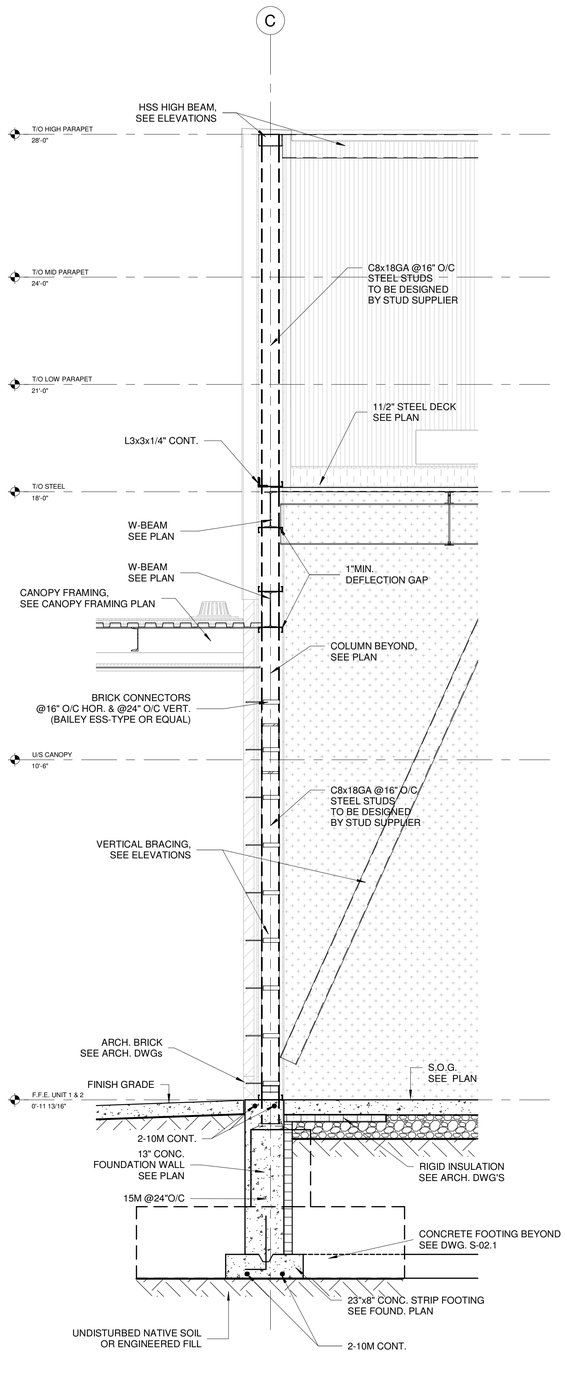
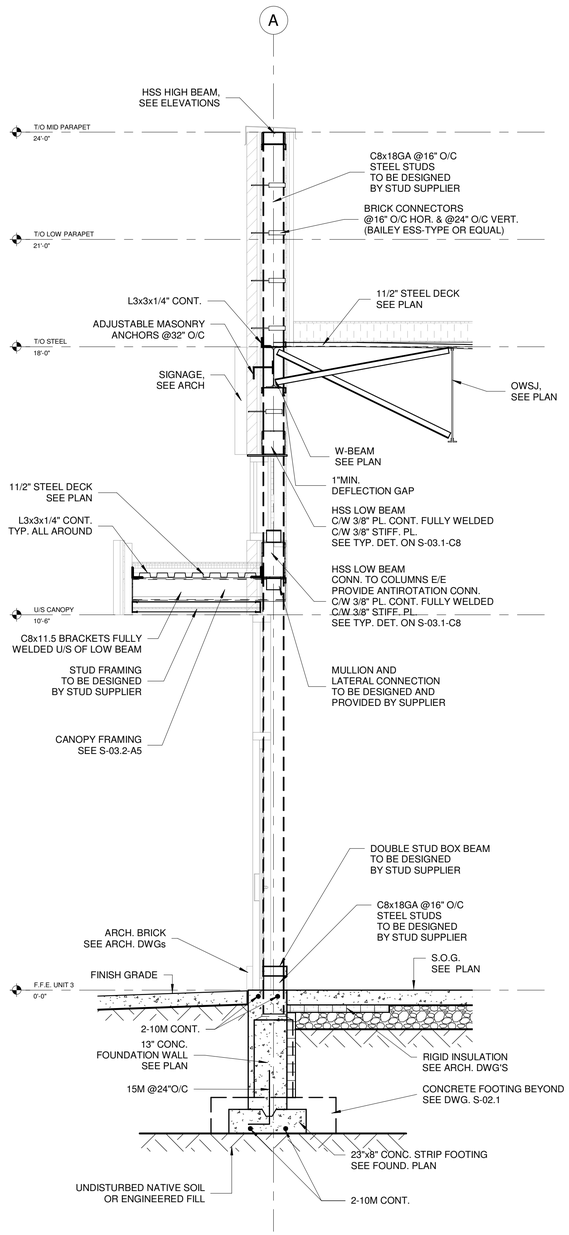


5 Section 5
S-05.2-C5 1/2" = 1'-0"



6 Section 6
S-05.2-C5 1/2" = 1'-0"



7 Section 7
S-05.2-C5 1/2" = 1'-0"

N

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

NO	DATE	DESCRIPTION	BY

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PROJECT
PROPOSED RETAIL BUILDING FOR WINDFIELDS FARMS
BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
SECTIONS

DRAWN BY: G.R.	DATE: 2023/03/13
CHECKED BY: S.Sv.	SCALE: 1/2" = 1'-0"
PROJECT NUMBER: 22-3849	DRAWING NUMBER: **S-05.2-C5**

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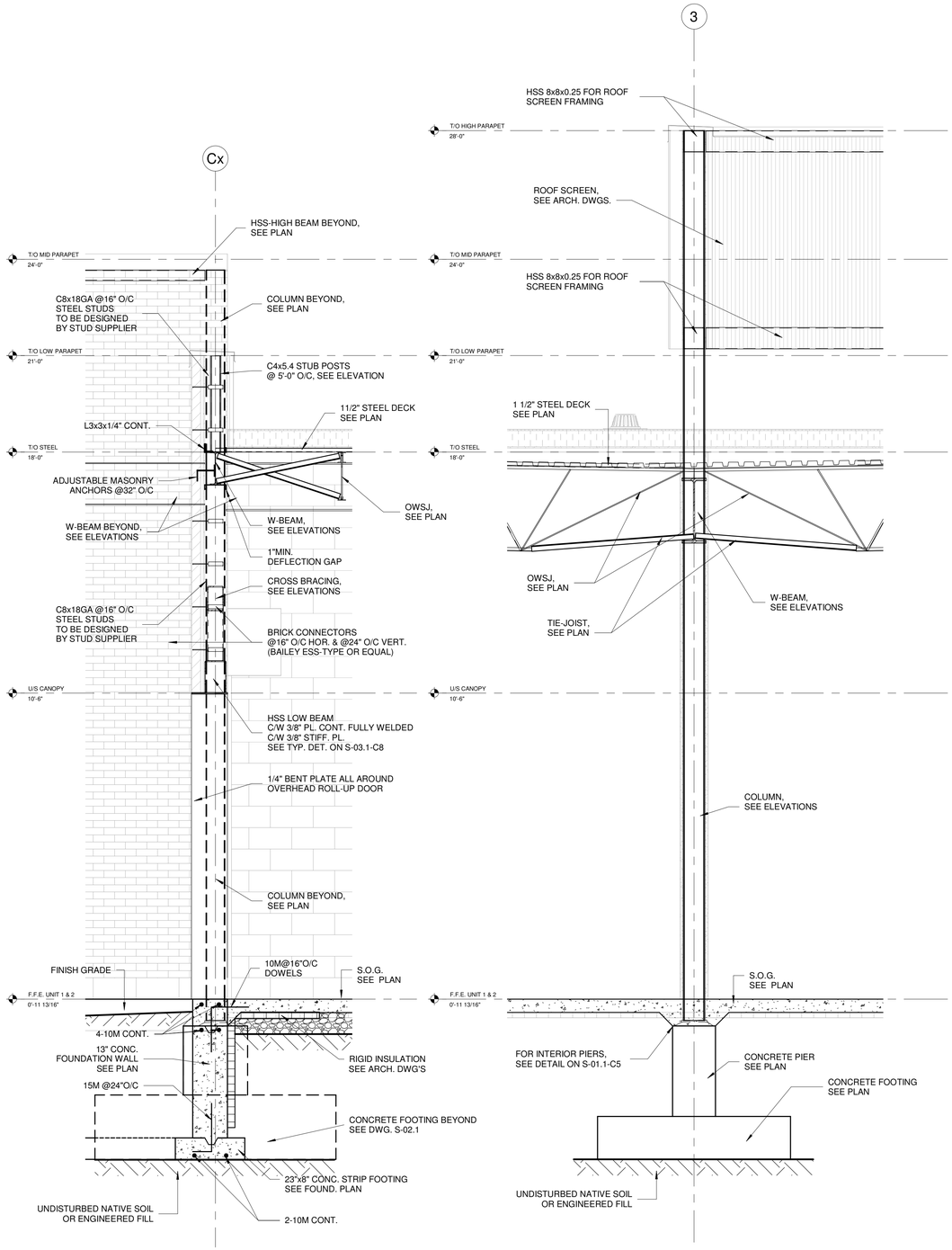

8 Section 8
S-05.3-C5 1/2" = 1'-0"

9 Section 9
S-05.3-C5 1/2" = 1'-0"

10 Section 10
S-05.3-C5 1/2" = 1'-0"

N

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3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

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DRAWING TITLE
SECTIONS

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	1/2" = 1'-0"
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-05.3-C5**

2023-07-14 1:00:20 PM

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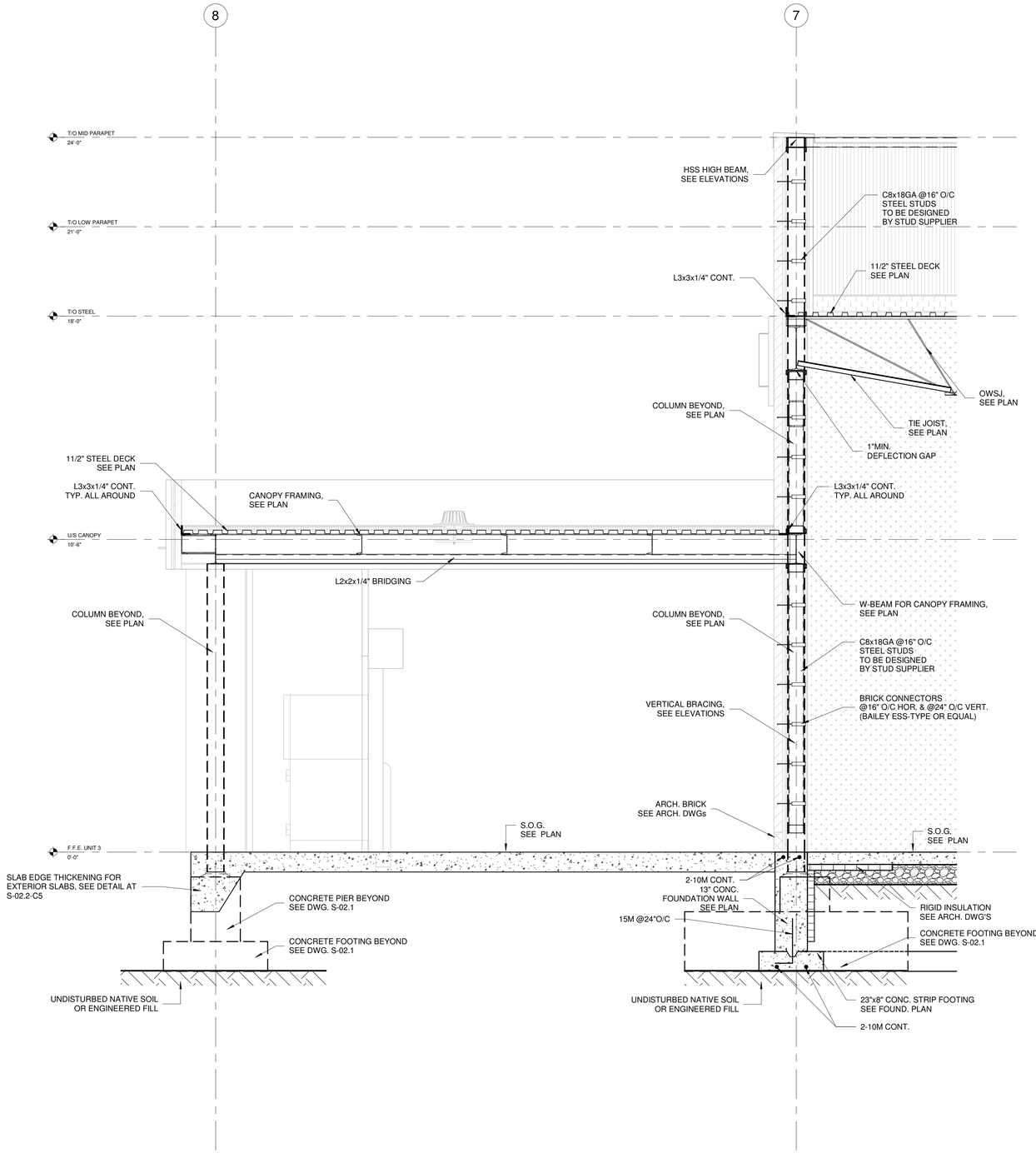

11 Section 11
S-05.4-C5 1/2" = 1'-0"

N

ISSUED

NO	DATE	DESCRIPTION	BY
1	2023-03-16	FOR 50% COORDINATION	S.Sv.
2	2023-06-15	FOR COORDINATION	S.Sv.
3	2023-06-16	FOR COORDINATION	S.Sv.
4	2023-07-14	FOR PERMIT & TENDER	S.Sv.

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BLOCK C2 PROPOSED BUILDING C5
2575 THOROUGHBRED ST., OSHAWA, ON. L1L0H4

DRAWING TITLE
SECTIONS

DRAWN BY	DATE
G.R.	2023/03/13
CHECKED BY	SCALE
S.Sv.	1/2" = 1'-0"
PROJECT NUMBER	DRAWING NUMBER
22-3849	**S-05.4-C5**

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